

CONSTRUCTION SPECIFICATIONS

**TECHNICAL SPECIFICATIONS
FOR
WQI WETLAND PROJECT**

CAL883416C

CALHOUN COUNTY, IOWA



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|  | <p>I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.</p> | |
| | <p> _____</p> <p>Meghan M. Funke, P.E.</p> | <p><u>11-22-2024</u> Date</p> |
| | <p>License number <u>27166</u></p> | |
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NATURAL RESOURCES CONSERVATION SERVICE CONSTRUCTION SPECIFICATION

IA-1 SITE PREPARATION

1. SCOPE

Site preparation work shall consist of clearing, grubbing, stripping, refuse removal, bank sloping and structure removal on the site as necessary to rid the site of all undesirable materials on or near the surface and prepare the site for the structure. All woody growth within the construction area shall be cleared and all stumps and roots one inch in diameter or larger shall be grubbed from the site. In addition, all areas within 25 feet of the footprint of the structure shall be cleared and grubbed, except as directed by the Engineer. The work shall also consist of the removal and disposal of structures (including fences) that must be removed to perform other items of work.

2. FOUNDATION PREPARATION

The construction areas shall be stripped a minimum of 6 inches to remove all unsuitable materials such as organic matter, grasses, weeds, sod, debris, and stones larger than 6 inches in diameter.

In an earth embankment foundation area, all channel banks and sharp breaks shall be sloped to no steeper than 1.5 horizontal to 1 vertical.

The foundation area shall be thoroughly scarified before placement of fill material. The surface shall have moisture added or shall be compacted if necessary, so that the first layer of fill material can be compacted and bonded to the foundation.

3. STRIPPED MATERIAL DISPOSAL

Stripped materials shall be buried, removed from the site, or disposed of as directed by the owner or Engineer. Whenever possible, material shall not be disposed of in the pool area created by the structure.

Stockpiled materials around a construction site should be placed so as not to hinder subsequent construction operations.

4. DISPOSAL OF REFUSE MATERIALS

Waste materials from clearing and structure removal shall be burned or buried at locations approved by the owner. Buried materials shall be covered with a minimum of 2 feet of earthfill. Whenever possible, material shall not be disposed of in any pool area created by the structure.

All refuse shall be disposed of in a manner which complies with all local and state regulations.

5. SALVAGE

Items to be salvaged shall be as shown on the drawings. Structures and fencing materials that are designated to be salvaged shall be carefully removed and neatly placed in the specified storage areas.

6. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in conformance with this specification and the construction details therefore are:

1. **Site Stripping & Preparation - Bid Item No. 1**
 - (a) This item will consist of work necessary to begin construction including, but not limited to, removing and disposing of existing vegetation (non-trees/brush) on areas to be excavated or filled.
 - (b) Site stripping will generally be limited to the existing waterway and non-cropped areas as most of the project area is currently cultivated field.
 - (c) Payment for Site Stripping will be made on a lump sum basis.
 - (d) Subsidiary Item: Refuse & Debris Removal, IA-1
2. **Crop Damage - Bid Item No. 24**
 - (a) It is anticipated that there will be damages to planted row crops as a result of this project. Damages to row crops are generally anticipated to occur within defined construction easements. The owner has negotiated a unit price with the landowner and that unit price has been included in the proposal. No changes to the unit price will be allowed. The intent of this bid item is to provide a flexible means for landowner reimbursement of an unknown quantity through the contractor. The contractor is encouraged to limit damages to the minimal extent possible.
 - (b) Payment for Crop Damage will be made on a per acre basis measured to the nearest 0.1 acres.
 - (c) Any crop damage outside the construction easements will be the sole responsibility of the contractor to reimburse the landowner, unless directed by the engineer.
3. Subsidiary Item, Refuse and Debris Removal
 - (a) This item will consist of all work to remove, dispose of, or bury refuse and debris within the easement area.

NATURAL RESOURCES CONSERVATION SERVICE CONSTRUCTION SPECIFICATION

IA-5 POLLUTION CONTROL

1. SCOPE

The work shall consist of installing measures or performing work to control erosion and minimize the production of sediment and other pollutants to water and air during construction operations.

2. MATERIALS

All materials furnished shall meet the requirements shown on the drawings or in the specifications.

3. EROSION AND SEDIMENT CONTROL MEASURES AND WORKS

The measures and works shall include, but are not limited to, the following:

Staging of Earthwork Activities: The excavation and moving of soil materials shall be scheduled so that areas unprotected from erosion will be minimized. These areas will be unprotected for the shortest time feasible.

Seeding: Structures and disturbed areas shall be seeded as soon as possible after construction is completed.

Temporary seeding may be used as an alternative to other stabilization measures as approved by NRCS.

Mulching: Construction areas that have been disturbed but have no construction activity scheduled for 21 days or more shall have erosion protection measures applied by the 14th day. This erosion protection may be mulching or other approved temporary measures. Construction areas shall not be left open during a winter shutdown period and shall be protected by mulching.

All seeding and mulching shall be completed in accordance with the seeding plan and Iowa Construction Specification IA-6, Seeding and Mulching for Protective Cover.

The following works may be temporary. If they are installed as a temporary measure, they shall be removed and the area restored to its original state when they are no longer needed or when permanent measures are installed.

Diversions: Diversions may be required to divert clean runoff water away from work areas and to collect runoff from work areas for treatment and safe disposition.

Stream Crossings: Culverts or bridges may be required where construction equipment must cross streams.

Sediment Basins: Sediment basins may be required to settle and filter out sediment from eroding areas to protect properties and streams below the construction site.

Sediment Filters: Straw bale filters, geotextile sediment fences, or other equivalent methods may be used to trap sediment from areas of limited runoff. Sediment filters shall be properly anchored to prevent erosion under them.

Waterways: Waterways may be required for the safe removal of runoff from fields, diversions, and other structures or measures.

4. CHEMICAL POLLUTION

The Contractor shall provide watertight tanks or barrels or construct a sump sealed with plastic sheets to be used to dispose of chemical pollutants, such as drained lubricating or transmission oils, greases, soaps, concrete mixer wash water, asphalt, etc., produced as a by-product of the construction work. At the completion of the construction work, sumps shall be removed and the area restored without causing pollution.

Sanitary facilities such as chemical toilets or septic tanks shall not be placed adjacent to live streams, wells, or springs. They shall be located at a distance sufficient to prevent contamination of any water sources. At the completion of construction work, facilities shall be disposed of without causing pollution.

5. AIR POLLUTION

The burning of brush or trash, or disposal of other materials shall adhere to local and state regulations.

Fire prevention measures shall be taken to prevent the start or the spreading of wild fires, which result from project work. Fire breaks or guards shall be constructed at locations shown on the drawings.

All public access or haul roads used by the contractor during construction of the project shall be sprinkled or otherwise treated to fully suppress dust. All dust control methods shall insure safe operations at all times. If chemical dust suppressants are used, the material shall be a commercially available product specifically designed for dust suppression and the application shall follow manufacturer's requirements and recommendations. A copy of the product data sheet and manufacturer's recommended application procedures shall be provided to the Engineer five working days before use.

6. MAINTENANCE, REMOVAL, AND RESTORATION

All pollution control measures and works shall be adequately maintained in a functional condition as long as needed during the construction operation. All temporary measures shall be removed and the site restored to as near original conditions as practical.

7. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in conformance with this specification and the construction details therefore are:

1. Subsidiary Item, Pollution Control
 - (a) This item shall consist of applying and performing all construction activities in a manner that will minimize water pollution, air pollution and soil erosion.
 - (b) No separate payment will be made for Pollution Control. Compensation for this item will be included in the payment Mobilization.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-6 SEEDING AND MULCHING FOR PROTECTIVE COVER

1. SCOPE

The work shall consist of seeding, mulching, and fertilizing all disturbed areas and other areas as indicated on the drawings or otherwise designated.

2. SEEDBED PREPARATION AND APPLICATION

The entire area to be seeded shall be reasonably smooth and all washes and gullies shall be filled to conform to the desired cross-section before actual seedbed preparation is begun. At this stage of the operation, the required fertilizer and lime shall be applied uniformly and incorporated into the top 3 inches of the soil with suitable tillage equipment. The seedbed preparation operation shall be suspended when the soil is too wet or too dry. The seedbed shall be loosened to a depth of at least three inches.

On side slopes steeper than 2-1/2 horizontal to 1 vertical, the 3-inch minimum depth of seedbed preparation is not required, but the soil shall be worked enough to insure sufficient loose soil to provide adequate seed cover.

Unless otherwise specified, the seeding operation shall be performed immediately after preparation of the seedbed. The seed shall be drilled or broadcast by equipment that will insure uniform distribution of the seed.

3. MATERIALS

~~REMOVED: The seeding, fertilizing, and mulching requirements are as specified on Form IA-GPA-4.~~

Straw from cereal grains or hay will be used as mulching material. It shall be relatively free of weeds.

4. MULCH APPLICATION

The required mulching shall be performed as soon as possible after seeding unless otherwise specified. The mulch shall be applied uniformly over the area. The type and rate shall be as specified. When mulching is required, all areas seeded during any one day shall be mulched within 24 hours. The mulch may be spread by any means that results in a uniform cover.

The mulch shall be anchored. Anchoring of the mulch may be performed by a mulch anchoring tool or regular farm disk weighted and set nearly straight, by installation of mulch netting, or by other methods approved by Engineer.

5. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in conformance with this specification and the construction details therefore are:

1. **Structure and Channel Seeding - Bid Item No. 2**

- a) This item will consist of seeding the dike except the upstream side slope below the weir elevation, auxiliary spillway, the tile outlet channel side slopes and any other disturbed areas noted on the plans or as determined by engineer.
- b) All seed must be clean and weed free. Seeding rates are expressed in bulk pounds per acre. Seed quality shall not drop below 70% Pure Live Seed (PLS) where PLS = (percent germination plus percent dormant seed) times percent purity.
- c) Seeding rates are as follows:
 - Smooth Brome grass 25 pounds/acre
- d) Seed shall be applied with a drill and placed at ¼ to ½ inch deep.
- e) Fertilizer shall be applied on the entire seeding area at the following rate:
 - Nitrogen (N) 30 pound/acre
 - Phosphorus (P₂O₅) 30 pounds/acre
 - Potassium (K₂O) 40 pounds/acre
- f) Straw mulch shall be applied at a rate of 2 tons per acre on all areas receiving structure and channel seeding.
- g) Seeding shall be completed during the following seeding periods:
 - Spring March 1 to May 15
 - Summer August 1 to September 15
 - Fall November 15 to Freeze-upIf construction is completed during any other time of the year, the seeding shall be performed at the next seeding period.
- h) If seeding is completed during the spring seeding period, a companion crop of oats shall be seeded at a rate of 1-1/2 bushels per acre.
- i) Measurement and Payment: For items of work for which specific unit prices are established in the contract, each area treated is measured as specified in this specification section and the area calculated to the nearest 0.1 acre. Payment for treatment is made at the contract unit price for designated treatment, which will constitute full compensation of the work. Measurement will be based on the areas successfully seeded.

2. **Buffer Seeding - Bid Item No. 3**

- a) This item will consist of seeding the areas designated on the plans as buffer seeding and include borrow areas, disturbed areas not seeded as part of structural seeding, and other areas within the easement. Buffer seeding is not required in areas below normal pool elevation established by the weir elevation.
- b) Some areas of the site may have existing CRP vegetation or steep slopes with existing vegetation. Local NRCS personnel will determine if these areas will be included as part of the buffer seeding areas for this project or will be left as is. This may affect the bid quantity and Contractor will verify with Engineer the number of acres that will require buffer seeding.
- c) All seed must be clean and weed free. Seeding rates are expressed in pounds of pure live seed per acre. All seed must be yellow-tagged Iowa ecotype.
- d) Seeding mixture shall include a minimum of 5 native grasses and 10 native forbs. The mixture shall provide a minimum of 30 grass seeds per square foot and 10 forbs seeds per square foot. Number of seeds will be based on

Iowa Conservation Practice 327 "Native Species for Wildlife". Contractor's proposed seed mix shall be submitted to Engineer and local NRCS office for approval at least 2 weeks before seed is to be applied.

- e) Seeding shall be completed during the following seeding periods:
 - Spring April 1 to June 30
 - Fall November 15 to Freeze-up
- f) The seed bed shall be properly prepared prior to seeding:
 - i. Any weed control measures shall be completed prior to seeding. If spraying is used, then a span of two weeks shall be allowed between spraying and seeding.
 - ii. If the land was in soybeans, no additional tillage is required. If the land was in corn or other vegetation, areas to be seeded shall be disked to thoroughly loosen and pulverize the soil to a depth of 3 inches. This may require multiple passes of equipment. If the land was used for pasture and has a smooth surface, the preparation in non-disturbed areas to be seeded shall include mowing any vegetation taller than 12 inches and applying an appropriate herbicide at the labeled rates to emergent growth 2 to 4 weeks after mowing. After the vegetation has died, the area shall be disked thoroughly loosen and pulverize the soil depth of 3 inches. If emergent growth occurs prior to seeding, the areas shall receive a second application of herbicide. Seeding shall not occur until the existing vegetation has died (about 1 week).
 - iii. If deeper disking is used at the site, a lighter disk or spring harrow shall be used to remove deep furrows.
 - iv. After disking operations and prior to seed application, the seedbed shall be firmed with a cultipacker or similar piece of equipment.
 - v. No lime or fertilizer is to be applied.
- g) Sow seed with contour using a grassland or rangeland drill set for the specified seeding rates. The drill shall be equipped with double coulter furrow openers. The drill shall be subject to acceptance by Engineer. Overlap each successive seeding pass to ensure complete coverage.
- h) Plant seed not more than 1/4 inch deep; some seed may be seen on the surface after seeding.
- i) Broadcasting by centrifugal-type or hydroseeder broadcasters, or by hand shall be allowed in areas not accessible to drills or other equipment. Once broadcast, the seed must be covered with soil to a depth no greater than 1/4 inch by means of hand rakes or other approved methods.
- j) Upon completion of the seeding operation, cultipack the seedbed to provide a positive seed-soil contact. If the drill seeder is equipped with an approved cultipacker or press wheels, separate operations shall not be necessary. The type of cultipacker/seeder to be used shall be subject to acceptance by Engineer.
- k) No mulch shall be required.
- l) Measurement and Payment: For items of work for which specific unit prices are established in the contract, each area treated is measured as specified in this specification section and the area calculated to the nearest 0.1 acre. Payment for treatment is made at the contract unit price for designated treatment, which will constitute full compensation of the work. Measurement will be based on the areas successfully seeded.

3. Subsidiary Item, Weed Control

- a) Weed control may be needed in portions of this site depending upon the start date of the contract, the initiation of grading, and the seeding dates.
- b) Weed control will be added to the contract with a change order to be negotiated between Contractor and Division based on conditions observed and the type of weed control used and will be paid only once. If delays require additional weed control, this will be paid for at Contractor's own expense.
- c) Weed control may include placement of a cover crop such as oats or rye, spraying with appropriate chemicals, or disking. If thistles are present, only spraying is allowed for weed control and shall include appropriate chemicals designed to control thistles.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

**CS-8 MOBILIZATION AND
DEMobilIZATION**

1. SCOPE

The work consists of the mobilization and demobilization of the contractor's forces and equipment necessary for performing the work required under the contract. It does not include mobilization and demobilization for specific items of work for which payment is provided elsewhere in the contract. Mobilization will not be considered as work in fulfilling the contract requirements for commencement of work.

2. EQUIPMENT AND MATERIALS

Mobilization shall include all activities and associated costs for transportation of contractor's personnel, equipment, and operating supplies to the site; establishment of offices, buildings, and other necessary general facilities for the contractor's operations at the site; premiums paid for performance and payment bonds including coinsurance and reinsurance agreements as applicable; and other items specified in section 4 of this specification.

Demobilization shall include all activities and costs for transportation of personnel, equipment, and supplies not required or included in the contract from the site; including the disassembly, removal, and site cleanup of offices, buildings, and other facilities assembled on the site specifically for this contract.

This work includes mobilization and demobilization required by the contract at the time of award. If additional mobilization and demobilization activities and costs are required during the performance of the contract as a result of changed, deleted, or added items of work for which the contractor is entitled to an adjustment in contract price, compensation for such costs will be included in the price adjustment for the item or items of work changed or added.

3. PAYMENT

Payment will be made as the work proceeds, after presentation of paid invoices or documentation of direct costs by the contractor showing specific mobilization and demobilization costs and supporting evidence of the charges of suppliers, subcontractors, and others. When the total of such payments is less than the lump sum contract price, the balance remaining will be included in the final contract payment. Payment of the lump sum contract price for mobilization and demobilization will constitute full compensation for completion of the work.

Payment will not be made under this item for the purchase costs of materials having a residual value, the purchase costs of materials to be incorporated in the project, or the purchase costs of operating supplies.

4. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

1. **Mobilization - Bid Item No. 4**

- (a) This item shall consist of mobilizing and demobilizing personnel and equipment in preparation to perform the work within the scope of this contract.
- (b) Any work that is necessary to provide access to the site including, but not limited to, grading, temporary culverts, rock installation and removal, and clearing will be included in this item. When construction is completed access areas will be restored, as close as practical, to its original condition.
- (c) The Contractor shall exercise caution to minimize the amount of damage caused by the grading and clearing operations.
- (d) Portable toilets shall be provided at the construction site and used for the sanitary facilities.
- (e) This item shall not include transportation of personnel, equipment and operating supplies within the work limits areas of this contract.
- (f) Subsidiary Item: Pollution Control IA-5.
- (g) Subsidiary Item: Sign Installation CS-8.

2. Subsidiary Item, Sign Installation

- (a) This item will consist of furnishing and installing the project sign, per the direction of the Iowa Department of Agriculture & Land Stewardship representative. Refer to standard detail for sign installation for how to install the project sign.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

**IA-9 SUBSURFACE DRAIN INVESTIGATION,
REMOVAL AND REPAIR**

1. SCOPE

The work shall consist of investigation, location, repair, and/or removal of subsurface drains (tile) near new or existing animal waste storage facilities or in wetland restoration, enhancement, or creation project areas, or other situations where subsurface drains may be present.

2. INVESTIGATION AND LOCATION

An inspection trench at least 10 inches wide shall be dug at the location shown on the drawings or as directed by the engineer or his representative. The trench shall be at least 6 feet deep measured from the original ground line, unless otherwise shown on the plans. The Engineer or his representative shall examine the trench and excavated material to identify tile lines.

Size, material, operating condition and direction of flow of each conduit shall be documented. Location and flow line elevation of each conduit shall be surveyed with horizontal and vertical control based on benchmarks shown on the plans.

The inspection trench shall be documented by surveying the natural ground and trench bottom location and elevations at the beginning, end, and every 50 feet for trenches longer than 50 feet.

Backfilling shall not be started without approval of the Engineer. See Section 5 for backfill specifications.

Trench shields, shoring and bracing, or other methods necessary to safeguard the workers and work, and to prevent damage to the existing improvements shall be furnished, placed, and subsequently removed by the contractor

3. TILE REPAIR

Unless designated for removal, replace damaged conduit with new conduit having equal or greater capacity using material specified in Section 6 or 7. When replacing short sections of clay or concrete tile with single-wall corrugated polyethylene pipe, use the next larger nominal size.

Make connections with manufactured fittings and tight joints. Where joints have gaps that would allow soil to enter, cover the joint with a permanent type material such as coal tar pitch treated roofing paper, fiber glass sheet or mat, or plastic sheet.

If the investigation trench has been excavated below the existing drain grade, backfill the trench with gravel or well-pulverized soil in layers not over four (4) inches thick and tamp by hand or manually directed power tamper to provide a firm foundation for the conduit at the existing grade. Do not backfill with any soil containing broken tile fragments.

Using selected soil free of hard clods, rocks, or frozen soil, hand tamp the backfill material around the haunch of the pipe in layers not over four (4) inches thick to provide support. Hold the conduit in place mechanically while placing excavated material around and over the conduit to ensure proper alignment and grade is maintained. Complete the backfill operation according to Section 5.

4. TILE REMOVAL

Remove conduits as shown on the plans or directed by the Engineer or his representative, including envelope filter material or other flow enhancing material when present.

Cap or plug the open ends of the disconnected conduit to prevent soil entry when the conduit will continue to function downstream, or otherwise shown on the plans. For a minimum distance of two feet around each sealed conduit end, backfill in layers not over four (4) inches thick and tamp by hand or manually directed power tamper to a density equal to or greater than the surrounding undisturbed soil. Do not backfill with any soil containing broken tile fragments, large stones, frozen material, or large dry clods.

Where tile are located beneath an existing animal waste facility, remove the tile or fill the entire length of tile with concrete or Portland cement grout as shown on the plans. When tile removal is specified, the owner shall contact the Iowa Department of Natural Resources (IDNR) for permission to remove the drainage tile under the structure. The structure shall be emptied of waste or lowered to a point below the tile prior to its removal. The structure must be retested for percolation and the results submitted to IDNR and approval received prior to reusing the structure.

If shown on the plans or directed by the engineer, reroute upstream drain lines so the capacity of the upstream drainage system is maintained. Install conduit in accordance with Iowa Construction Specification IA-46, Tile Drains for Land Drainage.

5. BACKFILL

Compact soil around disturbed tile as specified in Section 3 (Tile Repair) and Section 4 (Tile Removal). Keep the backfill within 5 feet of the conduit free from large stones, frozen material, and large dry clods. Unless otherwise shown on the plans, backfill the remainder of the trench as follows:

For trenches located under or near structures, backfill in 12 inch layers and compact each layer to a density equal to or greater than the surrounding undisturbed soil.

For other locations, backfill the remainder of each trench with the excavated soil material which shall extend above the ground surface and be well rounded over the trench.

6. MATERIALS

Unless otherwise shown on the plans, conduit and fittings used for repair shall conform to the specifications listed in Table 1. Perforated pipe shall have a water inlet area of at least 1 square inch per foot, provided by perforations spaced uniformly along the long axis of the pipe. The perforations shall be circular or slots. Circular perforations shall not exceed 3/16 inch in diameter. Slots shall not be more than 1/8 inch wide.

Table 1. Acceptable pipe for subsurface drain repair

| Kind of Pipe [#] | Specification |
|---|--------------------------|
| Corrugated Polyethylene (PE) Pipe and Fittings, 3 to 6 inch | ASTM F 405 |
| Corrugated Polyethylene (PE) Pipe and Fittings, 3 to 24 inch | ASTM F 667 |
| Corrugated Profile Wall (Dual Wall) Polyethylene (PE) pipe, 2 to 60 inch | ASTM F 2648 [§] |
| Corrugated Profile Wall (Dual Wall) Polyethylene (PE) pipe, 12 to 60 inch | ASTM F 2306 [§] |
| Polyvinyl Chloride (PVC) Plastic Pipe, Schedules 40, 80 and 120 | ASTM D 1785 |
| PVC Pressure-Rated Pipe (SDR Series) | ASTM D 2241 |
| Clay drain tile | ASTM C 4 |
| Concrete drain tile | ASTM C 412 |

[#]Pipe sizes are nominal and ranges are inclusive

[§]Pipe conforming to AASHTO M252 (3 to 10 inch), or AASHTO M294 (12-60 inch) is acceptable

7. SPECIAL SPECIFICATIONS

A. Measurement and Payment

Compensation for any work item described in the contract documents but not listed in the bid schedule will be included in the payment for the item of work to which it is made subsidiary. Such items and bid items to which they are made subsidiary are identified in Items of Work and Construction Details section of this specification.

For items of work which lump sum prices are established in the contract, the quantity of work will not be measured for payment. Payment for each item will be made at the contract lump sum price and will constitute full compensation for completion of the work.

For items of work for which specific unit prices are established in the contract, the payment will be made based on the approved quantity measured by the engineer or weight tickets. Payment will constitute full compensation of the work completed as defined by each work item.

B. Items of Work and Construction Details

1. Drain Tile Investigation and Removal - Bid Item No. 5

- (a) This item will consist of the excavation necessary to locate and remove all tile under the embankment and to locate and determine the tie-in locations for the tile locations shown on the plans. We do not anticipate

needing to remove tile found that does not cross under the embankment. This item shall also include backfilling of the trenches.

- (b) This item does not include the additional excavation required to excavate the embankment core trench. Excavation of the core trench is covered under Specification IA-21, Excavation.
- (c) The extent of the tile investigation shall be as shown on the drawings. If extra work is required to locate additional tile not shown on the map, the Contractor can request additional compensation, but has to be agree to by the Division.
- (d) The investigation should reveal where the tile crosses the embankment footprint or where it is located if it does not cross the embankment footprint
- (e) Payment for Drainage Tile Investigation and removal shall be made on a lump sum basis.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-11 REMOVAL OF WATER

1. SCOPE

The work shall consist of the removal of surface water and ground water as needed to perform the required construction in accordance with the plans and specifications.

2. DIVERTING SURFACE WATER

The Contractor shall build, maintain and operate all cofferdams, channels, diversions, flumes, sumps, and other temporary protective works needed to divert surface water away from the construction site while construction is in progress.

3. DEWATERING THE CONSTRUCTION SITE

Foundations, cutoff trenches, borrow areas and other parts of the construction site shall be dewatered as needed for proper execution of the construction work. The Contractor shall furnish, install, operate and maintain all works and equipment needed to perform the dewatering.

4. EROSION AND POLLUTION CONTROL

Removal of water from the construction site, including the borrow areas shall be accomplished in such a manner that erosion and the transmission of sediment and other pollutants are minimized.

5. REMOVAL OF TEMPORARY WORKS

After temporary works have served their purposes and before the Contractor leaves the site, they shall be removed.

6. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in conformance with this specification and the construction details therefore are:

1. Subsidiary Item, Removal of Water
 - (a) This item shall include all costs to divert, pump, dam, or other means to dewater the site.
 - (b) No separate payment will be made for Removal of Water. Compensation for this item shall be included in the payment for Excavation.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-21 EXCAVATION

1. SCOPE

The work shall consist of the excavation required by the drawings and specifications and disposal of the excavated materials. The cutoff trench and any other required excavations shall be dug to the lines and grades shown on the drawings or as staked in the field. Structure or trench excavations will conform to all safety requirements of OSHA.

2. USE OF EXCAVATED MATERIALS

Suitable materials from the specified excavations shall be used in the construction of required permanent earth fill. The suitability of materials for specific purposes shall be determined by the Engineer.

3. DISPOSAL OF WASTE MATERIAL

All surplus or waste material shall be disposed of in areas shown on the drawings or as approved by the NRCS Inspector. The waste material shall be smoothed and sloped to provide drainage.

4. STRUCTURE AND TRENCH EXCAVATION

Structure or trench excavations will conform to all safety requirements of OSHA.

5. BORROW EXCAVATION

When the quantities of suitable materials obtained from specified excavations are insufficient to construct the specified fills, additional materials shall be obtained from the designated borrow areas as shown on the drawings or as approved by the Engineer and the landowner. On wetland projects, borrow shall not be taken from the wetland area within 10 feet of the embankment or as shown on the drawings.

Borrow areas shall be excavated and grading completed in a manner to eliminate steep or unstable side slopes or hazardous or unsightly conditions.

6. OVER-EXCAVATION

Excavation beyond the specified lines and grades shall be corrected by filling the resulting voids with compacted earthfill, except that if the earth is to become the subgrade for riprap, sand or gravel bedding or drainfill, the voids shall be filled with material conforming to the specifications for the riprap, bedding or drainfill, as appropriate.

7. SPECIAL SPECIFICATIONS

A. MEASUREMENT AND PAYMENT

Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in this specification Section.

B. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in conformance with this specification and the construction details therefore are:

1. **Excavation, Bid Item No. 7**

- (a) This item will consist of excavation and finish grading of the pool area, channels, core trench, sediment basins, auxiliary spillway and any other miscellaneous items required to complete the project.
- (b) This item includes the hauling of excavated materials to either be used as earthfill or spoiled.
- (c) This bid item does not include the excavation and/or removal of topsoil.
- (d) Excavated material of sufficient quality, as determined by the engineer, shall be used as earthfill material for the construction of the embankments. It is anticipated that nearly all of the non-topsoil excavated material shall be suitable for earthfill.
- (e) Excess excavated materials shall be disposed of in the designated spoil areas.
- (f) Measurement and payment for Excavation shall be on a plan "P" cubic yard basis. Plan basis, designated with a "P" in the proposal means that the plan quantity listed in the proposal will be used to measure and pay for the bid item regardless of the actual quantity.
- (g) Subsidiary Item, Removal of Water IA-11
- (h) Subsidiary Item, Borrow Excavation IA-21
- (i) Subsidiary Item, Structure Excavation IA-21

2. Subsidiary Item, Borrow Excavation

- (a) This item will consist of borrowing additional suitable material from excavated and borrow areas shown on the drawings as needed to construct the embankment, berms and fill areas.
- (b) Borrow from any other area will not be allowed unless directed and or approved by the Engineer. Borrow will be allowed in the wetland pool area only where indicated on the drawings and as approved by the Engineer.
- (c) The topsoil from the borrow area shall be removed to a minimum depth of six (6) inches and stockpiled. When the borrow operations have been completed, grades shall be returned to that indicated on the plans and the topsoil shall be uniformly spread over the entire borrow area to depth of six (6) inches.
- (d) No Separate payment will be made for borrow excavation or topsoil spreading. Compensation for this item will be included in the payment for Excavation.

3. Subsidiary Item, Structure Excavation

- (a) This item shall consist of the excavation necessary to install the aluminum toe wall outlet structure, riprap, CMP, corrugated metal pipe tile outlets, drawdown structure, and riser inlet structure in the

locations and as shown on the drawings.

- (b) No separate payment will be made for Structure Excavation. Compensation for this item will be included in payment for Excavation.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-23 EARTHFILL

1. SCOPE

The work shall consist of the construction of earth fills required by the drawings and specifications. The completed work shall conform to the lines, grades, and elevations shown on the drawings or as staked in the field.

2. MATERIALS

All fill materials shall be obtained from required excavations and designated borrow areas. Fill materials shall contain no sod, brush, roots or other bio-degradable materials. Rocks larger than 6 inches in diameter shall be removed prior to compaction of the fill.

3. FOUNDATION PREPARATION

Foundations for earthfill shall be stripped a minimum of 6 inches to remove vegetation and other unsuitable materials. Foundation surfaces shall be scarified to a minimum depth of 2 inches.

Foundation and abutment surfaces shall not be sloped steeper than 1.5 horizontal to vertical unless otherwise shown on the drawings.

4. PLACEMENT

Fill shall not be placed until the required excavation and foundation preparation have been completed and the foundation has been inspected and approved by Engineer or his representative. Fill shall not be placed upon a frozen surface, nor shall snow, ice, or frozen material be incorporated in the fill.

Adjacent to structures or pipes, fill shall be placed in a manner which will prevent damage. The height of the fill adjacent to structures or pipes shall be increased at approximately the same rate on all sides.

The materials used throughout the earth fill shall be essentially uniform. Selective placement shall be as shown on the drawings or approved by NRCS.

If the surface of any layer becomes too hard and smooth for proper bond with the succeeding layer, it shall be scarified to a depth of not less than 2 inches before the next layer is placed.

The top surfaces of embankments shall be maintained approximately level during construction, except that a cross-slope of approximately 2% shall be maintained to ensure effective drainage.

When moving fill material from the borrow area(s) to the embankment by use of bulldozers only, the following steps shall be followed:

- Immediately after the borrow material is pushed to the embankment, it shall be spread in horizontal lifts placed parallel to the centerline of the embankment.

- Compactive effort will then be applied by operating equipment parallel to the centerline of the fill or embankment.
- Lift thickness shall in strict compliance with Clause 6 below.

Sectional fills are not allowed unless they are shown on the construction drawings.

5. CONTROL OF MOISTURE CONTENT

The moisture content of the fill material shall be adequate for obtaining the required compaction. Material that is too wet shall be dried to meet this requirement, and material that is too dry shall have water added and mixed until the requirement is met.

The moisture content of the fill material shall be such that a ball formed with the hands does not crack or separate when struck sharply with a pencil and will easily ribbon out between the thumb and finger.

Earth foundations under and adjacent to concrete structures shall be prevented from drying and cracking before concrete and backfill are placed.

The application of water to the fill materials shall be accomplished at the borrow areas insofar as possible.

6. COMPACTION

Earth fill shall be compacted by one of the following methods as specified on the plans. If no method is specified, compaction will be in accordance with Method 1.

Method 1 - Earthfill shall be placed so that the wheels or tracks of the loaded, hauling equipment, traveling in a direction parallel to the centerline of fill, pass over the entire surface of the layer being placed. Low ground pressure vehicles shall not be used for this purpose.

Method 2 - Two (2) complete passes of a tamping-type roller will be made over each layer. The roller shall be capable of exerting a minimum of two hundred (200) pounds per square inch.

Method 3 - Minimum density shall be 90% of the maximum density as determined by ASTM D 698 and as shown on the plans.

The maximum thickness of a lift of fill before compaction shall be 9 inches, unless otherwise indicated on the drawings

Fill adjacent to structures, pipe conduits, and anti-seep collars shall be placed in layers not more than 4 inches thick and compacted to a density equivalent to that of the surrounding fill. Methods used to obtain compaction from fine or coarse grained materials are as follows:

- For fine-grained materials, hand tamping or manually directed power tampers may be used. Hand tamping only shall be used to compact the earthfill under the bottom half of circular pipes. Manually directed power tampers shall not be used in tight spaces where applying full compactive effort will result in direct contact of

the tamper plate with the pipe. Care should be taken so that compaction around the spillway pipe does not cause uplift of the pipe resulting in a void beneath the pipe.

- For coarse grained materials (sands and gravels), vibratory plate compactors shall be used for obtaining compaction. However, hand tamping shall be used to compact the material under the bottom half of circular pipes.

In all cases, follow manufacturer instructions for the specific compaction equipment being used. Heavy equipment shall not be operated within 2 feet of any structure or pipe.

Compacting of fill adjacent to structures shall not be started until the concrete is 7 days old.

7. ISLANDS, MOUNDS, AND LOAFING AREAS ON WETLAND RESTORATION, ENHANCEMENT, OR CREATION PROJECTS

Islands shall be randomly located within the wetland area at locations shown on the drawings or as staked in the field. The orientation of island shorelines shall be random with attention given to prevailing winds to limit wave damage. In general, the side of the island with the longest dimension shall be parallel to the prevailing wind direction. Side slopes of islands shall be as shown on the drawings, but in no case shall be steeper than 6 horizontal to 1 vertical. Island shapes shall be irregular.

Loafing areas shall be constructed in the areas shown on the drawings or as staked in the field and shall be graded to drain runoff water. The elevation of at least one loafing area should be above the maximum water level whenever possible.

Excavated material not suitable for embankments, wetland dikes, or islands can be used to create mounds or blended into surrounding topography to create a natural appearance. Spoil material shall not be spread on existing wetland areas.

Organic soils shall not be used to construct islands, loafing areas, dikes, or embankments.

8. SPECIAL SPECIFICATIONS

A. Measurement and Payment

For items of work which specific unit prices are established in the contract, the volume of earthfill will be computed to the nearest cubic yard by the method of average cross-sectional end areas. No deduction in volume will be made for embedded items, such as, conduits inlet structures and their appurtenances. The pay limits for computation shall be as shown on the drawings with the further provisions that earthfill voids resulting from over excavation of the foundation, outside specified lines and grades, will be included in the measurement for payment only under the following conditions:

- Where such over excavation is directed by the engineer to remove unsuitable material, and

- Where the unsuitable condition is not a result of the contractor's improper construction operations as determined by the engineer.

Earthfill beyond the specified lines and grades to backfill excavation required for compliance with OSHA requirements will be considered subsidiary to the earthfill bid item(s).

Payment for each type and compaction class of earthfill and earth backfill is made at the contract unit price for that type and compaction class of earthfill. Such payment will constitute full compensation for all labor, material, equipment, and all other items necessary and incidental to the performance of the work.

Compensation for any item of work described in the contract, but not listed in the bid schedule is included in the payment for the item of work to which it is made subsidiary. Such items and the items to which they are made subsidiary are identified in this specific section.

B. Items of Work and Construction Details

Items of work to be performed in conformance with this specification and the construction details therefore are:

1. **Earthfill – Bid Items No. 8, No. 9, and No. 10**

- (a) This item shall consist of the earthfill necessary to construct the embankment, submerged berms, adjacent fill areas, backfill of the core trench excavation, and additional fill to be placed over the tile to provide adequate cover. Cohesive material found during general grading and designated borrow locations can be used for these areas. Sand and gravel found on site shall not be used for this earthfill.
- (b) Compaction shall be Method 2.
- (c) Rocks larger than 6" shall be removed prior to compaction.
- (d) Measurement and payment for Earthfill shall be on a plan "P" cubic yard basis. Plan basis, designated with a "P" in the proposal means that the plan quantity listed in the proposal will be used to measure and pay for the bid item regardless of the actual quantity.
- (e) The plan quantities listed reflect the constructed, in-place compacted volumes.
- (f) Payment for these items shall be based on plan quantity unless additional length or location of tile varies from what is shown on the plans. The plan quantity is based on neat lines and does not account for any shrinkage.
- (g) Payment will constitute full compensation for the following related Subsidiary items: Pollution Control, Removal of Water, and Backfill Required Excavation.

2. Subsidiary Item, Backfill Required Excavation

- (a) This item shall consist of backfilling the areas excavated to install the other components related to the project such as piping or structures and to locate and remove the tile lines.
- (b) Compaction adjacent to the structures shall be as indicated above. All other compaction shall be Method 1 or equivalent.
- (c) No separate payment will be made for Backfill of Structure Excavation. Compensation for this item will be included in payment for Drain Tile, Corrugated Metal Pipe; Drawdown Structure; Riser Inlet Structure, and Tile Investigation & Removal.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-26 TOPSOILING

1. SCOPE

The work shall consist of salvaging topsoil from borrow areas or required excavations and spreading it on the exposed disturbed areas.

2. QUALITY OF TOPSOIL

Topsoil shall consist of friable surface soil reasonably free of grass, roots, weeds, sticks, stones, or other foreign materials.

3. EXCAVATION

After the site has been stripped, cleared, and grubbed, and the topsoil shall be removed from borrow areas and required excavation areas to the depth as shown on the drawings. Topsoil shall be stockpiled at locations approved by the Engineer.

4. SPREADING

Spreading shall not be done when the ground or topsoil is frozen, excessively wet, or otherwise in a condition detrimental to the work. Surfaces designated to be covered shall be lightly scarified just prior to the spreading operation. Where compacted fills are designated to be covered by topsoil, the topsoil shall be placed concurrently with the fill and shall be bonded to the compacted fill with the equipment.

Topsoil shall be placed to the minimum depth shown on the drawings. After the spreading operation is completed, the surface shall be finished to a reasonably smooth surface.

5. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

1. Topsoiling – Bid Item No. 11

- (a) This item will consist of stripping, salvaging, stockpiling and spreading salvaged (stockpiled) topsoil as the surface layer of all excavations and earthfills that will be seeded.
- (b) Areas to receive a minimum four (4) inch layer of topsoil include: the upstream surface of the embankment above normal pool, the downstream surface of the embankment, borrow area outside of normal pool, the tops and backslopes of designated spoil piles placed adjacent to excavated channels and as also directed on the plans
- (c) Areas to receive a minimum six (6) inch layer of topsoil include: the upstream surface of the structure below normal pool, and any grading or borrow areas inside of normal pool.
- (d) Measurement and payment for Topsoiling shall be on a plan "P" cubic yard basis. Plan basis, designated with a "P" in the proposal means that the plan quantity listed in the proposal will be used to measure and pay for the bid item regardless of the actual quantity.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-31 CONCRETE

1. SCOPE

The work shall consist of furnishing, forming, placing, finishing, and curing Portland cement concrete including steel reinforcement.

2. MATERIALS

Portland Cement shall conform to ASTM C 150 and shall be Type I or Type II.

Fine Aggregates shall conform to ASTM C 33 and shall be composed of clean, uncoated grains of material.

Coarse Aggregates shall be gravel or crushed stone conforming to ASTM C 33 and shall be clean, hard, durable and free from clay or coating of any character. The maximum size of coarse aggregate shall be 1 1/2 inches or as shown on the drawings.

Water shall be clean and free from injurious amounts of oil, acid, salt, alkali, organic matter, or other deleterious substances.

Air entraining agent shall conform to ASTM C 260.

Fly ash may be used as a partial substitution for Portland cement and shall be in strict compliance with ASTM C 618, Class F or C. The loss by ignition shall not exceed 4.0 percent.

Blast-furnace slag may be used as a partial substitution for Portland cement and shall be in conformance with ASTM C 989 for ground granulated blast-furnace slag (GGBF slag).

Water-reducing admixtures shall conform to ASTM C 494 and may be the following types:

1. Type A - Water-reducing admixture
2. Type D - Water-reducing and retarding admixture
3. Type F - Water-reducing, high range admixture (superplasticizer).
4. Type G - water-reducing, high range, and retarding admixture (superplasticizer).

Type D or G admixture may be used when the air temperature is over 80 degrees F. at the time of mixing and/or placement.

Calcium Chloride or other antifreeze compounds or accelerators will not be allowed.

Preformed expansion joint filler shall be a commercially available product made of bituminous, sponge rubber or closed cell foam materials with a minimum thickness of 1/2 inch.

Reinforcing steel shall be free from loose rust, oil, grease, paint, or other deleterious matter. Reinforcing steel shall conform to one or more of the following:

1. Reinforcing Bars - ASTM A 615 or A 996, Grade 40 or greater, deformed.

2. Welded Wire Fabric - ASTM A 185 or A 497.

Waterstops shall be either metallic or nonmetallic. Metallic waterstops shall be fabricated from sheets of copper or galvanized steel. Nonmetallic waterstops shall be made of natural or synthetic rubber or vinyl chloride polymer or copolymer. Rubber, polymer and copolymer waterstops shall have ribbed or bulb-type anchor flanges and a hollow tubular center bulb, unless otherwise shown on the drawings. All waterstops shall be of the sizes shown on the drawings.

Curing compound shall be a liquid membrane-forming compound suitable for spraying on the concrete surface. The curing compound shall meet the requirements of ASTM C 309 Type 2 (white pigmented).

3. CONCRETE DESIGN MIX

The contractor will be responsible for the determining the design mix proportions in accordance with the requirements included in this paragraph and shall provide a copy of the mix to the NRCS Engineer at least 3 days prior to placing any concrete. The concrete mix shall be of such proportions as to provide a minimum strength of 3500 p.s.i. in 28 days, unless otherwise shown on the drawings. The air content shall be 4 to 8 percent of the volume of the concrete at the time of placement. The slump shall be 2 to 5 inches except when superplasticizer is used. The slump shall be 3 inches or less prior to the addition of superplasticizer admixture and shall not exceed 7 1/2 inches following addition and mixing. The fine aggregate shall be 30-50 percent of the total combined aggregate based on oven dry weights. The contractor shall provide tests to verify that the design mix meets the requirements. In lieu of this, one of the following mix proportions per cubic yard may be used:

| <u>Mix Number</u> | <u>Minimum Cement, Pounds</u> | <u>Fly Ash, Pounds</u> | <u>GGBF Slag, Pound</u> | <u>Maximum ** Water, Gallons</u> |
|-------------------|---------------------------------------|----------------------------|---------------------------------|--|
| 1 | 564 | 0 | 0 | 33 |
| 2 | 470 | 45-90 | 0 | 31-34 |
| 3 | 517 | 129 | 0 | 31 * |
| 4 | 366 | 114 | 91 | 31 * |
| 5 | 259 | 103 | 155 | 31 * |

** Total of available aggregate moisture, mixing water added at the plant and mixing water added at the job site (one gallon equals 8.33 pounds).

* Requires water reducing admixture.

4. MIXTURES AND MIXING

Ready-mixed concrete shall be batched, mixed and transported in accordance with ASTM C 94. Concrete shall be uniform and thoroughly mixed when delivered to the forms. No mixing water in excess of the amount shown for the design mix or in an amount that would cause the maximum slump to be exceeded shall be added to the concrete during mixing, hauling or after arrival at the point of delivery. The concrete shall be batched and mixed so that the temperature of the concrete at the time of placing shall be between 50 and 90 degrees F.

5. BATCH TICKET

The contractor shall obtain from the supplier a delivery ticket for each batch of concrete before unloading at the site. The following information shall be included on the ticket: name

of concrete supplier, job name or location, date, truck number, amount of concrete, time loaded or time of first mixing cement, aggregate, and mixing water added at the plant, type and amount of cement, type and amount of admixtures, oven dry weights of fine and coarse aggregate, and moisture content(%) or weight of water contained in the aggregates.

The following information shall be added to the batch ticket on site: mixing water added on site, time concrete arrived on site and time concrete was unloaded.

Upon completion of the concrete placement, copies of all batch tickets shall be provided to NRCS.

6. REINFORCING STEEL

Before reinforcement is placed, the surfaces of the bars or mesh shall be cleaned to remove any loose, flaky rust, mill scale, oil, grease, or other foreign substances. After placement, the reinforcement shall be maintained in a clean condition until it is completely embedded in the concrete.

Reinforcing bars shall be cut and bent according to ACI Standard 315.

Tack welding of bars shall not be permitted. Reinforcement shall be accurately placed as shown on the drawings and secured in position in a manner that will prevent its displacement during placement of concrete. Metal chairs, metal hangers, metal spacers or concrete chairs shall be used to support reinforcement. Precast concrete chairs shall be manufactured from concrete equal in quality to the concrete being placed. Precast concrete chairs shall be moist at the time concrete is placed

Splices of reinforcing bars shall be made only at the locations shown on the drawings, unless otherwise approved by the NRCS Engineer. All reinforcing splices and placement shall be in accordance with ACI 350 and shown on the drawings.

After placement of the reinforcement, concrete shall not be placed until the reinforcement has been inspected and approved by NRCS.

7. PREPARATION OF FORMS AND SUBGRADE

Prior to placement of concrete, the forms and subgrade shall be free of woodchips, sawdust, debris, water, ice, snow, extraneous oil, mortar, or other harmful substances or coatings. Any oil on the reinforcing steel or other surfaces required to be bonded to the concrete shall be removed. All surfaces shall be firm and damp prior to placing concrete. Placement of concrete on mud, dried earth, uncompacted fill, or frozen subgrade will not be permitted.

The forms and associated false-work shall be substantial and unyielding and shall be constructed so that the finished concrete will conform to the specified dimensions and elevations. Forms will be mortar tight. Forms with torn surfaces, worn edges, dents or other defects will not be used. Forms shall be coated with a nonstaining form release agent before being set into place. Excess form coating material shall not stand in puddles in the forms or come in contact with the steel reinforcement or hardened concrete against which fresh concrete is to be placed.

Form accessories to be partially or wholly embedded in the concrete, such as ties and hangers, shall be of a commercially manufactured type. Non fabricated wire shall not be used. Form ties shall be constructed so that the ends or end fasteners can be removed without causing spalling at the surface of the concrete.

Metal form ties used within the forms on structures with a total volume of concrete exceeding fifteen cubic yards shall be equipped with cones or other devices that permit their removal to a depth of at least one inch without damage to the concrete. The holes resulting from cones and other devices shall be patched in accordance with Section 9.

Form ties except those specifically covered by the preceding paragraph shall be broken off flush with the formed surface. Any surface areas which have been spalled or otherwise damaged shall be repaired in accordance with Section 9.

Steel tying and form construction adjacent to new concrete shall not be started until concrete has cured at least 12 hours.

Concrete joints shall be of the type and at the locations shown on the drawings.

Splices in metal waterstops shall be brazed, welded or overlapped and bolted. Splices in nonmetallic waterstops shall be cemented or joined as recommended by the manufacturer.

8. PLACING CONCRETE

Concrete shall not be placed until the subgrade, forms, and steel reinforcement have been inspected and approved by the NRCS Inspector. Any deficiencies are to be corrected before the concrete is delivered for placement.

Concrete shall be delivered to the site and discharged into the forms within 1 1/2 hours after the introduction of the cement to the aggregates. When a superplasticizer is used, the concrete shall be discharged within the manufacturer's recommended time limit for discharge after addition of the admixture. In hot weather or under conditions contributing to quick setup of the concrete, discharge of the concrete shall be accomplished in 45 minutes unless a set-retarding admixture is used, in which case the manufacturer's recommended time limit will apply.

Addition of water at the job site may be done at the beginning of placement of each load of concrete in order to obtain allowable slump, provided that the maximum water content and water/cement ratio in the design mix is not exceeded. Addition of water will not be permitted after placement of the load has started.

The concrete shall be deposited as closely as possible to its final position in the forms and shall be worked into corners and around reinforcement and other embedded items in a manner which prevents segregation. Formed concrete shall be deposited in layers 24 inches or less in depth and shall be continuously deposited so that no concrete will be deposited on concrete which has hardened sufficiently to cause the formation of "cold joints". Concrete containing superplasticizer shall be placed in lifts not exceeding 5 feet in depth. If the surface layer of concrete sets during placement to the degree that it will not flow and merge with the succeeding layer when tamped or vibrated, the contractor shall discontinue placing concrete and install a construction joint. Construction joints shall be completed as shown on the drawings or by one of the following methods:

1. The joint shall be constructed using a 6 inch wide by 1/4 inch steel plate. The surfaces of the construction joint shall be prepared by washing and scrubbing with a wire brush or wire broom to expose coarse aggregate. The steel plate shall be embedded 3" in the concrete.
2. The joint surface shall be cleaned to expose coarse aggregate by sandblasting or air-

water cutting after the concrete has gained sufficient strength to prevent displacement of the coarse aggregate or cement fines. The surface of the concrete shall not be cut so deep as to undercut the coarse aggregate. The joint shall be washed to remove all loose material after cutting.

The surfaces of all construction joints shall be kept continuously moist for at least 1 hour prior to placement of the new concrete. The new concrete shall be placed directly on the cleaned and washed surface. New concrete shall not be placed until the hardened concrete has cured at least 12 hours.

Concrete shall not be dropped more than 5 feet vertically unless suitable equipment is used to prevent segregation. Concrete containing superplasticizer shall not be dropped more than 12 feet vertically.

Immediately after the concrete is placed in the forms, it shall be consolidated by vibration, spading or hand tamping as necessary to insure smooth surfaces and dense concrete. Care should be taken not to over-vibrate concrete containing superplasticizer. Vibration shall not be supplied directly to the reinforcing steel, the forms or concrete which has hardened to the degree that it does not insure a monolithic bond with the preceding layer. The use of vibrators to transport concrete in the forms or conveying equipment will not be permitted.

9. FORM REMOVAL AND FINISHING

Forms shall be left in place for at least 24 hours after placing concrete. Forms shall be removed in such a way as to prevent damage to the concrete. Supports shall be removed in a manner that will permit concrete to take the stresses due to its own weight uniformly and gradually.

Immediately after removal of the forms, concrete which is honey combed, damaged or otherwise defective shall be repaired or replaced. All cavities or depressions resulting from form tie removal shall be patched with a non-shrink grout, mortar mix or epoxy-type sealer. Non-shrink grout consists of 1 part cement and 2-1/2 parts sand that will pass a No. 16 sieve. Only enough water shall be added to produce a filling which is at the point of becoming rubbery when the material is solidly packed.

All repaired and patched areas shall be cured as required in Section 10.

10. CURING

Concrete shall be cured for a period of not less than 7 consecutive days by one of the following approved methods:

- A. Membrane Curing: Concrete shall be cured with white pigmented curing compound. The compound shall be sprayed on moist concrete as soon as free water has disappeared, but shall not be applied to any surface until patching, repairs and finishing of that surface are completed. Curing compound shall not be applied to surfaces requiring bond to subsequently placed concrete, such as construction joints, shear plates, reinforcing steel, and other embedded items. Surfaces subjected to heavy rainfall or running water within 3 hours after curing compound has been applied or surfaces damaged by subsequent construction operations during the curing period, shall be reapplied in the same manner as the original application.
- B. Moist Curing: Concrete shall be cured by maintaining all surfaces continuously wet for the entire curing period.

- C. Cover: Adequately cover an exposed structure with burlap mats, or other material and continually soak with water.

11. BACKFILLING

Backfilling may begin when the curing period has ended. Backfill against the structure will be placed in no more than 4-inch layers and compacted by hand tamping or with manually directed power tampers or plate vibrators. Layers compacted in this manner shall extend not less than 2 feet from any part of the concrete structure.

12. HOT AND COLD WEATHER CONCRETING

When the atmospheric temperature may be expected to drop below 40° F. at the time concrete is delivered to the work site, during placement, or at any time during curing period, concrete shall be mixed, placed and protected in accordance with ACI Standard 306, "Recommended Practice for Cold Weather Concreting."

When climatic or other conditions are such that the temperature of the concrete may reasonably be expected to exceed 90° F. at the time of delivery to the work site, during placement or during the first 24 hours after placement, concrete shall be mixed, placed and protected in accordance with ACI Standard 305, "Recommended Practice for Hot Weather Concreting."

13. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in conformance with this specification and the construction details therefore are:

1. Subsidiary Item – Concrete
 - (a) This item shall consist of furnishing and placing poured concrete and reinforced concrete as shown on the drawings.
 - (b) No separate payment will be made for Concrete. Compensation for this item will be included in the payment for the Drawdown Structure and Riser Inlet Structure.

NATURAL RESOURCES CONSERVATION SERVICE CONSTRUCTION SPECIFICATION

IA-46 TILE DRAINS FOR LAND DRAINAGE

1. SCOPE

The work shall consist of furnishing and installing drainage pipe (tubing) and tile and the necessary fittings and appurtenances.

2. MATERIALS

Concrete drain tile shall conform to the requirements of ASTM C 412 and clay drain tile shall conform to the requirements of ASTM C 4.

Corrugated polyethylene (PE) pipe (tubing) and fittings shall conform to ASTM F 405 (3" to 6") or F 667 (3" to 24"), as appropriate. Corrugated profile wall (dual wall) polyethylene (PE) pipe shall meet or exceed the requirements of ASTM F 2648 (2" to 60") or ASTM F 2306 (12" to 60"). Pipe conforming to AASHTO M 252 (3" to 10"), or AASHTO M 294 (12" to 60") is acceptable. Perforated tubing shall have a water inlet area of at least 1 square inch per foot, provided by perforations spaced uniformly along the long axis of the tubing. The perforations shall be circular or slots. Circular perforations shall not exceed 3/16 inch in diameter. Slots shall not be more than 1/8 inch wide.

3. EXCAVATION

Unless otherwise specified, excavation for and subsequent installation of each drain line shall begin at the outlet end and progress upstream.

The trench or excavation for the conduit shall be constructed to the line, depth, cross section, and grade shown on the drawings, or as directed by the Engineer. The trench bottom shall be smooth and free of exposed rock. If rock is encountered in the trench bottom, over-excavate the trench and place at least 6 inches of compacted earth or sand bedding in the trench to bring it up to the conduit grade.

If not otherwise shown on the drawings, trench width at the top of the conduit shall be the minimum required to permit installation and provide bedding conditions suitable to support the load on the conduit, but with not less than three (3) inches of clearance on each side of the conduit. Maximum trench width shall be the conduit diameter plus 12 inches measured at the top of the conduit, unless approved bedding is installed.

Trench shields, shoring and bracing, or other methods, necessary to safeguard the workers and work, and to prevent damage to the existing improvements shall be furnished, placed, and subsequently removed by the contractor.

Plow installation is allowed. Minimum trench width shall be two (2) inches wider than the conduit on each side. Grade control and bedding conditions shall be closely inspected during plow installation. Boulders, cobbles, or cemented soils can cause the plow to jump and lose grade. These hardpoints can also puncture or dimple and deform the pipe.

4. PREPARING THE BEDDING

Unless otherwise specified, no filter or envelope is required. In stable soils, the bottom of the trench shall be shaped to form a semicircular, trapezoidal, or 90-degree "V" groove in its center. The groove shall be shaped to fit the size of tile. The 90-degree "V" groove shall not be used on conduits greater than 6 inches in diameter.

If the bottom of the trench does not provide a sufficiently stable or firm foundation for the drain tile, a sand-gravel mix or other approved materials shall be used to stabilize the bottom of the trench.

5. FILTER OR ENVELOPE MATERIAL

When a filter is specified, the shape of the bottom of the trench, gradation and the thickness of the filter or envelope material to be placed around the conduit will be as shown on the drawings. The envelope or filter material shall be placed in the bottom of the trench just prior to the laying of the conduit. The conduit shall then be laid and the envelope or filter material placed over the conduit.

6. PLACEMENT AND JOINT CONNECTIONS

All drains shall be laid to grade.

Joints between lateral concrete and clay drain tiles shall vary with soil type as follows:

- a. Peat and muck - 1/4 inch preferred, 3/8 inch maximum
- b. Clay - 1/8 inch preferred, 1/4 inch maximum
- c. Silt and loam - 1/16 inch preferred, 1/8 inch maximum
- d. Sand - tightest possible fit.

Joints between main drain tile, which serve only to collect and transport drainage water from lateral tile lines, should be the tightest fit possible.

Where the joint width exceeds the maximum above, the joint shall be covered with a permanent type material such as coal tar pitch treated roofing paper, fiber glass sheet or mat, or plastic sheet.

After placement and blinding of plastic tubing, but prior to backfilling, sufficient time shall elapse to allow the tubing to reach the ambient temperature of the trench. All split fittings shall be securely tied with nylon cord before backfill is placed. When corrugated plastic tubing is used, no more than 5% stretch will be allowed.

7. CONNECTIONS

Lateral connections will be made with manufactured appurtenances (wyes, tees, etc.) comparable in strength and durability with the specified conduit unless otherwise shown on the drawings.

Existing drain lines not shown on the drawings but encountered during installation shall be bridged across the trench or connected into the new line, as directed by the Engineer.

Connections with the outlet pipe shall be made watertight.

8. OUTLETS

A continuous section of non-perforated conduit at least 20 feet long shall be used at the outlet. At least two-thirds of the outlet pipe shall be buried in the ditch bank, and the cantilever section must extend to the toe of the ditch side slope or the side slope protected from erosion. Acceptable materials for use at the outlet include the following:

- a. Corrugated metal pipe, galvanized or aluminum, 16 gauge minimum;
- b. Smooth steel pipe with a minimum wall thickness of 3/16 inch;
- c. Smooth plastic pipe, polyvinyl chloride (PVC), with a SDR of 26 or less or schedule 40 or heavier; or
- d. Corrugated profile wall (dual wall) polyethylene pipe (PE).

All plastic (PVC) and polyethylene pipe (PE) outlets shall include ultra-violet stabilizer. PVC and PE pipe outlets shall not be used where burning vegetation on the outlet ditch bank is likely to create a fire hazard.

The outlet shall be equipped with a flap-gate type rodent guard.

9. BLINDING

After the conduit is placed in the excavated groove, friable material from the sides of the trench shall be placed around the conduit, completely filling the trench to a depth of not less than six (6) inches over the top of the conduit. For material to be suitable it must not contain hard clods, rocks, frozen soil, or fine material which will cause a silting hazard to the drain. Conduit placed during any one day shall be blinded by the end of the day's work.

10. BACKFILLING

The backfilling of the trench shall be completed as rapidly as consistent with the soil conditions. Automatic backfilling machines may be used. Backfill shall extend above the ground surface and be well rounded over the trench.

Unless otherwise shown on the plans, in mineral soils, the minimum depth of cover over subsurface drains shall be 2.4 feet. In organic soils, the minimum depth of cover after initial subsidence shall be 3.0 feet.

11. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in accordance with this specification and the construction details therefore are:

- 1. **6-Inch Drain Tile, Perforated Polyethylene - Bid Item No. 12**
 - (a) This item will consist of furnishing and installing 6-inch corrugated perforated polyethylene tubing used as drain tile as shown on the drawings.
 - (b) Outlets shall be corrugated metal pipe.
 - (c) Measurement and payment for 6-Inch Drain Tile, Perforated Polyethylene shall be on an installed linear foot basis; and shall include all necessary fittings, adaptors, and connections.

- 2. **12-Inch Drain Tile, Perforated Polyethylene - Bid Item No. 13**
 - (a) This item will consist of furnishing and installing 12-inch corrugated perforated polyethylene tubing used as drain tile as shown on the drawings.
 - (b) Outlets shall be corrugated metal pipe.
 - (c) Measurement and payment for 12-Inch Drain Tile, Perforated Polyethylene shall be on an installed linear foot basis; and shall include all necessary fittings, adaptors, and connections.

3. **4-Inch Toe Drain Tile, Perforated Polyethylene - Bid Item No. 14**

- (a) This item will consist of furnishing and installing 4-inch corrugated perforated polyethylene tubing used as toe drain tile as shown on the drawings.
- (b) Outlets shall be corrugated metal pipe.
- (c) Measurement and payment for 4-Inch Drain Tile, Perforated Polyethylene shall be on an installed linear foot basis; and shall include all necessary fittings, adaptors, and connections.

4. **Stop Log Storage Structure - Bid Item No. 20**

- (a) This item will consist of furnishing and installing the Stop Log Storage Structure as shown on the drawings, including the drainage stone, pipe, locking cover, and locks.
- (b) The Stop Log Storage Structure shall be constructed using 24-inch corrugated dual wall polyethylene tubing as shown on the drawings.
- (c) Measurement and payment for the Stop Log Storage Structure shall be on a Lump Sum basis.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-51 CORRUGATED METAL PIPE CONDUITS

1. SCOPE

The work shall consist of furnishing and placing circular, arched or elliptical corrugated metal pipe and the necessary fittings.

2. MATERIALS

Metallic-coated steel corrugated pipe and fittings shall be zinc-coated or aluminized, Type 2, and shall conform to the requirements of ASTM A 760 and A 929 for the specified type and size of pipe. Aluminum corrugated pipe shall conform to the requirements of ASTM B 745 for the specified type and size of pipe. All pipe is subject to the following additional requirements:

- A. When polymer coating is specified, pipe, coupling bands and anti-seep collars shall be coated in accordance with ASTM A 762. All riveted joints shall be caulked as described in paragraph B.
- B. Pipe with annular corrugations shall be furnished with caulked seams. Riveted pipe joints shall be caulked with a bituminous mastic material during fabrication to provide a watertight joint. All circumferential and longitudinal seams shall be caulked before riveting. This shall be accomplished by applying a uniform bead of the mastic compound to the inner lap surface before riveting such that when the rivets are in place, all voids are filled and a coating of mastic is between the lap surfaces. The inner surface of coupling bands shall be asphalt coated in the field prior to installation. A neoprene gasket having a minimum thickness of 3/8 inch and a minimum width of 7 inches may be used in lieu of mastic coated coupling bands.
- C. Welded or lock seams in helical corrugated pipe are considered to be watertight.
- D. When close riveted pipe is specified: (1) the pipe shall be fabricated so that the rivet spacing in the circumferential seams shall not exceed 3 inches, except that 12 rivets will be sufficient to secure the circumferential seams in 12-inch pipe, and (2) in those portions of the longitudinal seams that will be covered by the coupling bands, the rivets shall have finished flat heads or the rivets and holes shall be omitted and the seams shall be connected by welding to provide a minimum of obstruction to the seating off the coupling bands.
- E. Double riveting or double spot welding of pipe less than 42 inches in diameter may be required. If specified, the riveting or welding shall be done in the manner specified for pipe 42 inches or greater in diameter.

3. COUPLING BANDS

Coupling bands shall meet the requirements of the table below or have detailed drawings submitted for approval by the State Conservation Engineer. Coupling bands shall be of the same minimum thickness (gage) as the pipe being connected.

| Description of Coupling Band | Maximum Fill Height, Ft. | Maximum Pipe Diam., In. |
|---|--------------------------|-------------------------|
| 24-inch wide coupling band with four 1/2-inch Diam. galvanized rods with tank lugs for annular or helical corrugated metal pipe. Bands shall have a minimum lap of 3 inches. | All | All |
| Hugger band from Armco Steel Corp. for helical corrugated metal pipe with reformed ends; and for annular corrugated pipe. Bands include O-ring gaskets and two 1/2-inch Diam. galvanized rods and lugs. ^{1/} | 35 | 48 |
| Hugger band without rods and lugs but including O-ring gaskets. ^{1/} | 20 | 24 |
| Angles riveted or welded to a coupling band and drawn tight with bolts. Bands shall be a minimum of 7 corrugations wide and have a minimum lap of 2 inches. | 35 | 15 |
| Flanged couplings for helical corrugated pipe welded to the ends of the pipe and field assembled by a minimum of 3/8-inch Diam. bolts. A joint sealer shall be placed between the flanges to ensure water tightness. | 25 | 12 |

^{1/} Use is limited to sites where soft foundation and conduit elongation is not anticipated.

4. FABRICATION

Fabrication of all appurtenances shall be done as shown on the drawings. All appurtenances shall be made of metallic-coated steel when corrugated steel pipe is used and aluminum when used with aluminum pipe. Dissimilar metals shall not be installed in contact with each other.

5. REPAIR OF DAMAGED COATINGS

The Contractor shall place the pipe without damaging the pipe or coatings. The pipe shall be transported and handled in a manner to prevent damage to the pipe or coating.

Breaks, scuffs, or other damage to the various coatings shall be repaired as follows:

- A. Metallic Coating - by thoroughly wire brushing the damaged area and cleaning with solvent, and then painting two coats of one of the following paints:
 - (1) Zinc Dust - Zinc Oxide Primer conforming to ASTM D 79 and D 520.
 - (2) Single package, moisture cured urethane prime in silver metallic color.
 - (3) Zinc-rich cold galvanized compound, brush, or aerosol applications.
- B. Polymer Coating - apply two coats of polymer material similar to and compatible with the durability, adhesion and appearance of the original polymer coating. The

repair coating shall be a minimum thickness of 0.010 (10 mils) after drying and shall bond securely to the pipe.

6. LAYING AND BEDDING THE PIPE

The pipe shall be laid to the line and grade shown on the drawings and shall be firmly and uniformly bedded throughout its entire length. Details of the bedding are as shown on the drawings.

The pipe shall be laid with the outside laps of circumferential joints pointing upstream and with longitudinal laps on the sides at approximately the vertical mid-height of the pipe. Field welding of corrugated galvanized steel pipe will not be permitted. The pipe sections shall be joined with coupling bands.

7. BACKFILLING

Special care shall be taken during backfill operations not to disturb the grade and alignment.

The pipe shall be tied down or loaded sufficiently during backfilling around the sides to prevent its being lifted from the bedding.

Backfill material shall have sufficient moisture so that optimum compaction can be obtained. Backfill around the pipe shall be placed in layers not more than 4 inches thick before compaction.

Each layer of backfill shall be compacted with power tampers, hand tampers, or plate vibrators to the same density requirements as specified for the adjacent embankment. Backfill over and around the pipe shall be brought up uniformly on all sides. The passage of earth moving equipment will not be allowed over the pipe until backfill has been placed above the top of the pipe surface to a depth of two (2) feet.

8. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in accordance with this specification and the construction details therefore are:

1. 6-Inch Drain Tile Outlet - Bid Item No. 15

- (a) This item will consist of furnishing and installing the corrugated metal pipe tile outlets including animal guards as shown on the drawings.
- (b) The corrugated metal pipe shall be 16 gage aluminum coated with annular or helical corrugations.
- (c) Measurement and payment for 6-Inch Drain Tile Outlet shall be on an installed linear foot basis.

2. 12-Inch Drain Tile Outlet - Bid Item No. 16

- (a) This item will consist of furnishing and installing the corrugated metal pipe tile outlets including animal guards as shown on the drawings.

- (b) The corrugated metal pipe shall be 16 gage aluminum coated with annular or helical corrugations.
- (c) Measurement and payment for 12-Inch Drain Tile Outlet shall be on an installed linear foot basis.

3. **18-Inch Corrugated Metal Pipe (CMP) – Bid Item No. 17**

- (a) This item will consist of providing and installing the 18" CMP outlet piping as shown in the plans.
- (b) The corrugated metal pipe shall be closed riveted caulk seam (CRCS) round pipe with 2-2/3" x 1/2" annular corrugations and shall conform to ASTM A760 with polymer coating.
- (c) Measurement and payment for 18-Inch Corrugated Metal Pipe (CMP) shall be on an installed linear foot basis.
- (d) Subsidiary Item, CMP Aprons, 18-Inch, IA-51.
- (e) Subsidiary Item, CMP Bends 18-Inch, IA-51.
- (f) Subsidiary Item, CMP Water-Tight Coupling Bands, IA-51
- (g) Subsidiary Item, Drainage Diaphragm, IA-51.

4. **Drawdown Structure - Bid Item No. 18**

- (a) This item will consist of providing and installing the Drawdown Structure, stop logs, and concrete base as shown in the plans.
- (b) Provide detailed shop drawings of the Drawdown Structure and all appurtenances.
- (c) Contractor shall furnish two metal lifting rod with hook end and handles capable of lifting the supplied stop logs while standing on the surface above the drawdown structure.
- (d) Measurement and payment for Drawdown Structure shall be on a Lump Sum Basis.
- (e) Subsidiary Item, Concrete, IA-31.

5. **Riser Inlet Structure - Bid Item No. 19**

- (a) This item will consist of providing and installing the Riser Inlet Structure as shown on the drawings including the 36-inch riser, trash rack, and concrete base.
- (c) Provide detailed shop drawings of the riser inlet structure and appurtenances.
- (d) Measurement and payment for Riser Inlet Structure shall be on a Lump Sum Basis.
- (e) Subsidiary Item, Concrete, IA-31

6. Subsidiary Item, CMP Aprons, 18-Inch

- (a) This item will consist of furnishing and installing the 18-inch CMP aprons as shown on the drawings. The pipe apron shall include an animal guard, Agridrain RG-18 or equal.
- (b) No separate payment will be made for the CMP aprons. Compensation for this item shall be included in the payment for 18-Inch Corrugated Metal Pipe (CMP).

7. Subsidiary Item, CMP Bends, 18-Inch
 - (a) This item will consist of furnishing and installing the 18-inch CMP bends as shown on the drawings.
 - (b) No separate payment will be made for the CMP bends. Compensation for this item shall be included in the payment for 18-Inch Corrugated Metal Pipe (CMP).

8. Subsidiary Item, CMP Water-Tight Coupling Bands
 - (a) This item will consist of furnishing and installing the corrugated metal coupling bands for connections of all pipe sections and structure stubouts.
 - (b) No separate payment will be made for the CMP coupling bands. Compensation for this item shall be included in the payment for 18-Inch Corrugated Metal Pipe (CMP).

9. Subsidiary Item, Drainage Diaphragm
 - (a) This item will consist of constructing the filter diaphragm and filter outlet as shown in the plans.
 - (b) Filter material shall be ASTM C33 sand, hauled in from off-site.
 - (c) Filter diaphragm sand shall be placed uniformly in layers not to exceed 8 inches thick before compaction. Each layer shall be thoroughly wetted (flooded) immediately prior to compaction.
 - (d) Each layer of sand shall be compacted by a minimum of two passes of a vibratory plate compactor weighing at least 160 pounds. The compactor shall have a minimum centrifugal force of 2,450 pounds at a vibrating frequency of no less than 5,000 cycles per minute (or by a minimum of two passes of a vibratory smooth wheeled roller weighing at least 325 pounds with a centrifugal force of 2,250 pounds at a vibrating frequency of no less than 4,500 cycles per minute).
 - (e) The sand shall be placed to avoid segregation of particle sizes and to ensure the continuity and integrity of all zones. No foreign material shall be allowed to become intermixed with or otherwise contaminate the drainfill.
 - (f) Traffic shall not be permitted to crossover filter zones at random. Equipment crossovers shall be maintained, and the number and location of such crossovers shall be established and approved before the beginning of diaphragm placement. Each crossover shall be cleaned of all contaminating material and shall be inspected and approved by the engineer before the placement of additional drain fill material.
 - (g) Any damage to the foundation surface or the trench sides or bottom occurring during placement of sand filter shall be repaired before the sand filter zone placement is continued.
 - (h) The upper surface of the sand filter zone constructed concurrently with adjacent zones of earthfill shall be maintained at a minimum elevation of 1 foot above the upper surface of adjacent earthfill.
 - (i) No separate payment will be made for filter diaphragm. Compensation for this item shall be included in the payment for 18-Inch Corrugated Metal Pipe (CMP).

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-61 LOOSE ROCK RIPRAP

1. SCOPE

The work shall consist of the construction of loose rock riprap revetments, structures and blankets, including filter layers or bedding where specified.

2. MATERIALS

Rock for loose rock riprap, filter layers or bedding shall come from sources approved by NRCS. The rock shall be excavated, selected and handled as necessary to meet the quality and grading requirements of this specification and the construction drawings.

Individual rock fragments shall be dense, sound and free from cracks, seams and other defects conducive to accelerated weathering. The rock fragments shall be angular to sub rounded in shape. The least dimension of an individual rock fragment shall be not less than 1/3 the greatest dimension of the fragment unless otherwise specified on the construction drawings.

3. SUBGRADE PREPARATION

The subgrade surfaces on which the riprap or bedding course is to be placed shall be cut or filled and graded to the lines and grades shown on the drawings. When fill to subgrade lines is required, it shall consist of approved materials and shall be compacted to a density equal to the adjacent existing soil material.

Rock materials shall not be placed until the foundation preparation is completed and the subgrade surfaces have been inspected and approved by the Engineer.

4. EQUIPMENT-PLACED ROCK RIPRAP

Rock shall be placed by equipment on the surfaces and to the depths specified. The riprap shall be constructed to the full thickness in one operation and in such a manner as to avoid serious displacement of the underlying materials. The rock shall be delivered and placed in a manner that will ensure that the riprap in place shall be reasonably homogeneous with the larger rocks uniformly distributed and firmly in contact, one to another with the smaller rocks and spalls filling the voids between the larger rocks.

Riprap shall be placed in a manner to prevent damage to structures. Hand placing will be required to the extent necessary to prevent damage to adjacent structures.

5. HAND-PLACED RIPRAP

Rock shall be placed by hand on the surfaces and to the depths specified. It shall be securely bedded with the larger rocks firmly in contact, one to another. Spaces between the larger rocks shall be filled with smaller rocks and spalls. Smaller rocks shall not be grouped as a substitute for larger rock. Flat slab rock shall be laid on edge unless otherwise specified.

6. FILTER LAYERS OR BEDDING

When the drawings specify filter layers or bedding beneath riprap, the filter or bedding material shall be spread uniformly on the prepared subgrade surfaces to the depth specified. Compaction of filter layers or bedding will not be required, but the surface of such layers shall be finished reasonably free of mounds, dips or windrows.

7. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in accordance with this specification and the construction details therefore are:

1. Rock Riprap - Bid Item No. 22

- (a) This item shall consist of furnishing and placing Rock Riprap as shown on the drawings.
- (b) Rock Riprap shall meet the requirements of NRCS Material Specification 523, Rock Type 1.
- (c) Rock Riprap shall be IADOT Class E Revetment.
- (d) Rock Riprap shall meet the following gradation:

| Size (Inches) | Percent Passing |
|------------------|--------------------|
| 24 | 100 |
| 15 | 75 |
| 12 | 50 |
| 8 | 0 |

- (e) The riprap should be screened over an 8-inch grizzly to remove smaller material which will aid in the placement of riprap.
- (f) Measurement and payment for Rock Riprap shall be on a per ton basis, rounded to the nearest ton. Weigh tickets shall be provided to the Engineer for all Rock Riprap.
- (g) Subsidiary Item - Geotextile Fabric, IA-95

2. Erosion Stone - Bid Item No. 23

- (a) This item shall consist of furnishing and placing Erosion Stone as shown on the drawings
- (b) Erosion Stone shall meet the requirements of NRCS Material Specification 523, Rock Type 1.
- (c) Erosion stone (where called for) shall meet the following gradation:
 - i. Nominal 6-inch size
 - ii. 100% passing the 9" screen
 - iii. 100% retained on the 3" screen
- (d) Measurement and payment for Erosion Stone shall be on a per ton basis, rounded to the nearest ton. Weigh tickets shall be provided to the Engineer for all Erosion Stone.
- (e) Subsidiary Item - Geotextile Fabric, IA-95

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-81 METAL FABRICATION AND INSTALLATION

1. SCOPE

The work shall consist of furnishing, fabricating, and installing metalwork including metal parts of composite structures.

2. MATERIALS

Metal shall be of structural quality. Finished surfaces shall be smooth and true to assure proper fit. Bolts, nuts, washers, rods, rivets, etc., shall be of a material equal to the metal being fastened.

3. PROTECTIVE COATINGS

Protective coatings will consist of either galvanizing or painting and shall be applied by the fabricator.

Galvanizing shall consist of a zinc coating by the hot dip process, except that bolts, nuts, and washers may have an electrodeposited zinc coating.

Paint System for this specification shall consist of the application of one coat of Epoxy Polyamide Primer (lead and chromate free) and one or more coats of Epoxy Polyamide (intermediate or finish), lead free. When finished, it will have a minimum dry film thickness of 8.0 mils.

4. FABRICATION

Materials shall be carefully fabricated as shown on the drawings. The fabrication shall be smooth and true to assure proper fit. Galvanized items shall not be cut, welded, or drilled after the zinc coating is applied.

5. ERECTION

The metal shall be erected true and plumb, closely conforming to the drawings.

6. SPECIAL SPECIFICATIONS

A. ITEMS OF WORK AND CONSTRUCTION DETAILS

Items of work to be performed in accordance with this specification and the construction details therefore are:

1. Aluminum Toe Wall Outlet Structure – Bid Item No. 6

- (a) This item will consist of furnishing and installing the aluminum toe wall outlet structure shown on the drawings. This item shall include field cutting metal to the specified angles and slopes shown on the drawings.
- (b) Contractor must submit shop drawings to engineer for review.
- (c) The aluminum toe wall outlet structure shall have the requirements as shown in the plans.
- (d) Measurement and payment of the Aluminum Toe Wall Outlet Structure shall be on a lump sum basis. Payment shall include all

equipment, labor, materials, and costs to complete the work to furnish and install the aluminum toe wall outlet structure as shown in the plans.

**NATURAL RESOURCES CONSERVATION SERVICE
CONSTRUCTION SPECIFICATION**

IA-95 GEOTEXTILE

1. SCOPE

This work shall consist of furnishing all materials, equipment, and labor necessary for the installation of geotextile.

2. MATERIAL QUALITY

Geotextiles shall be manufactured from synthetic long chain or continuous polymeric filaments or yarns, having a composition of at least 95 percent, by weight, of polypropylene, polyester or polyvinylidene-chloride. The geotextile shall be formed into a stable network of filaments or yarns that retain their relative position to each other, are inert to commonly encountered chemicals and are resistant to ultraviolet light, heat, hydrocarbons, mildew, rodents and insects. Unless otherwise specified, the class and type of geotextile shall be as shown on the drawings and shall meet the requirements for materials that follow:

- a. Woven Geotextile shall conform to the physical properties listed in Table 1. The woven geotextile shall be manufactured from monofilament yarns that are woven into a uniform pattern with distinct and measurable openings. The geotextile shall be manufactured so that the yarns will retain their relative position with regard to each other. The yarns shall contain stabilizers and/or inhibitors to enhance their resistance to ultraviolet light or heat exposure. The edges of the material shall be selvaged or otherwise finished to prevent the outer yarn from unraveling.
- b. Nonwoven Geotextile shall conform to the physical properties listed in Table 2. Nonwoven geotextiles shall be manufactured from randomly oriented fibers that have been mechanically bonded together by the needle-punched process. In addition, one side may be slightly heat bonded. Thermally bonded, nonwoven geotextiles, in addition to mechanically bonded, nonwoven geotextiles, may be used for Road Stabilization. The filaments shall contain stabilizers and/or inhibitors to enhance their resistance to ultraviolet light or heat exposure.
- c. The geotextile shall be shipped in rolls wrapped with a protective covering to keep out mud, dirt, dust, debris and direct sunlight. Each roll of geotextile shall be clearly marked to identify the brand, type and production run.

3. STORAGE

Prior to use, the geotextile shall be stored in a clean dry place, out of direct sunlight, not subject to extremes of either hot or cold, and with the manufacturer's protective cover in place. Receiving, storage, and handling at the job site shall be in accordance with the requirements in ASTM D 4873.

4. SURFACE PREPARATION

The surface on which the geotextile is to be placed shall be graded to the neat lines and grades as shown on the drawings. The surface shall be reasonably smooth and free of loose rock and clods, holes, depressions, projections, muddy conditions and standing or flowing water (unless otherwise on the drawings).

5. PLACEMENT

Prior to placement of the geotextile, the soil surface will be inspected for quality assurance of design and construction. The geotextile shall be placed on the approved prepared surface at the locations and in accordance with the details shown on the drawings. The geotextile shall be unrolled along the placement area and loosely laid (not stretched) in such a manner that it will conform to the surface irregularities when material is placed on or against it. The geotextile may be folded and overlapped to permit proper placement in the designated area.

The geotextile shall be joined by overlapping a minimum of 18 inches (unless otherwise specified), and secured against the underlying foundation material. Securing pins, approved and provided by the geotextile manufacturer, shall be placed along the edge of the panel or roll material to adequately hold it in place during installation. Pins shall be steel or fiberglass formed as a "U", "L", or "T" shape or contain "ears" to prevent total penetration. Steel washers shall be provided on all but the "U" shaped pins. The upstream or up-slope geotextile shall overlap the abutting down-slope geotextile. At vertical laps, securing pins shall be inserted through both layers along a line through approximately the midpoint of the overlap. At horizontal laps and across slope laps, securing pins shall be inserted through the bottom layer only. Securing pins shall be placed along a line approximately 2 inches in from edge of the of the placed geotextile at intervals not to exceed 12 feet unless otherwise specified. Additional pins shall be installed as necessary and where appropriate, to prevent any undue slippage or movement of the geotextile. The use of securing pins will be held to the minimum necessary. Pins are to be left in place unless otherwise specified.

Should the geotextile be torn or punctured, or the overlaps disturbed, as evidenced by visible geotextile damage, subgrade pumping, intrusion, or grade distortion, the backfill around the damaged or displaced area shall be removed and restored to the original approved condition. The repair shall consist of a patch of the same type of geotextile being used, overlaying the existing geotextile. The patch shall extend a minimum of 2 feet from the edge of any damaged area.

The geotextile shall not be placed until it can be anchored and protected with the specified covering within 48 hours or protected from exposure to ultraviolet light. Geotextile shall be placed in accordance with the following application specifications according to the use indicated on the drawing.

Slope protection - Class I or II as indicated in Tables 1 and 2.

The geotextile shall not be placed until it can be anchored and protected with the specified covering within 48 hours or protected from exposure to ultraviolet light. Rock shall not be pushed or rolled over the geotextile.

Class I, unprotected - limit height for dropping stone onto bare geotextile to 3 feet.

Class II, protected - require the use of 6 inches a clean pit-run gravel over the geotextile to cushion the stone and limit the height of drop to 3 feet.

On slopes with strong seepage flow, the geotextile must be in intimate contact with the soil to prevent erosion of the soil surface. Use 6 inches of a clean pit-run gravel over the geotextile to hold it in place and minimize voids under the riprap. Embedment of the geotextile in a trench to form a cutoff at regular intervals down the slope will prevent erosion under the fabric. Place cutoffs more closely together in highly erodible soils and wider apart in more stable soils.

Subsurface drains - Class III as indicated in Tables 1 and 2.

The geotextile shall not be placed until drainfill or other material can be used to provide cover within the same working day. Drainfill material shall be placed in a manner that prevents damage to the geotextile. In no case shall material be dropped on uncovered geotextile from a height of more than 5 feet.

Road stabilization - Class IV as indicated in Tables 1 and 2.

The geotextile shall be unrolled in a direction parallel to the roadway centerline in a loose manner permitting conformation to the surface irregularities when the roadway fill material is placed on its surface. In no case shall material be dropped on uncovered geotextile from a height of more than 5 feet. Unless otherwise specified, the minimum overlap of geotextile panels joined without sewing shall be 24 inches. The geotextile may be temporarily secured with pins recommended or provided by the manufacturer, but they shall be removed before the permanent covering material is placed.

6. SPECIAL SPECIFICATIONS

A. Measurement and Payment

Compensation for any work item described in the contract documents but not listed in the bid schedule will be included in the payment for the item of work to which it is made subsidiary. Such items and bid items to which they are made subsidiary are identified in Items of Work and Construction Details section of this specification.

For items of work which lump sum prices are established in the contract, the quantity of work will not be measured for payment. Payment for each item will be made at the contract lump sum price and will constitute full compensation for completion of the work.

For items of work for which specific unit prices are established in the contract, the payment will be made based on the approved quantity measured by the engineer or weight tickets. Payment will constitute full compensation of the work completed as defined by each work item.

B. Items of Work and Construction Details

1. **Subsidiary Item, Geotextile**

- (a) This item shall consist of furnishing and placing geotextile on all surfaces that contact the rock riprap as shown on the drawings and at concrete aprons.
- (b) Geotextiles shall conform to the requirements of IDOT Engineering Fabric for embankment erosion control.
- (c) The geotextile shall be placed with the long dimension parallel to the channel.
- (d) No separate payment will be made for geotextile. Compensation of this item will be included in the payment for the related bid item, Riprap.

TABLE 1. REQUIREMENTS FOR WOVEN GEOTEXTILES ^{1/}

| Property | Test Method | Class I | Class II | Class III | Class IV |
|---|----------------------------------|-------------|--------------|-------------|-------------|
| Grab tensile strength (pounds) | ASTM D 4632 | 247 minimum | 180 minimum | 180 minimum | 315 minimum |
| Elongation at failure (%) | ASTM D 4632 | <50 | <50 | < 50 | <50 |
| Trapezoidal tear strength (pounds) | ASTM D 4533 | 90 minimum | 67 minimum | 67 minimum | 112 minimum |
| Puncture strength (pounds) | ASTM D 6241 | 495 minimum | 371 minimum | 371 minimum | 618 minimum |
| Ultraviolet light (% retained strength) | ASTM D 4355 | 50 minimum | 50 minimum | 50 minimum | 70 minimum |
| Permittivity (sec ⁻¹) | ASTM D 4491 | | as specified | | |
| Apparent opening size (AOS) ^{2/} | ASTM D 4751 | | as specified | | |
| Percent open area (POA) (%) | USACE ^{3/} CWO-02215-86 | | as specified | | |

1/ All values are minimum average roll values (MARV) in the weakest principal direction, unless noted otherwise.

2/ Maximum average roll value.

3/ Note: CWO is a USACE reference.

TABLE 2. REQUIREMENTS FOR NONWOVEN GEOTEXTILES ^{1/}

| Property | Test Method | Class I ^{2/} | Class II ^{2/} | Class III ^{2/} | Class IV ^{2/} |
|--|-----------------------|-----------------------|------------------------------|-------------------------|------------------------|
| Grab tensile strength (pounds) | ASTM D 4632 grab test | 202 minimum | 157 minimum | 112 minimum | 202 minimum |
| Elongation at failure (%) ^{1/} | ASTM D 4632 | 50 minimum | 50 minimum | 50 minimum | 50 minimum |
| Trapezoidal tear strength (pounds) | ASTM D 4533 | 79 minimum | 56 minimum | 40 minimum | 79 minimum |
| Puncture strength (pounds) | ASTM D 6421 | 433 minimum | 309 minimum | 223 minimum | 433 minimum |
| Ultraviolet light (retained strength) (%) | ASTM D 4355 | 50 minimum | 50 minimum | 50 minimum | 50 minimum |
| Permittivity sec ⁻¹ | ASTM D 4491 | | 0.70 minimum or as specified | | |
| Apparent opening size (AOS) (mm) ^{3/} | ASTM D 4751 | | 0.22 maximum or as specified | | |

1/ All values are minimum average roll values (MARV) in the weakest principal direction, unless otherwise noted.

2/ Needle punched geotextiles may be used for all classes. Heat-bonded or resin-bonded geotextiles may be used for Class IV only.

3/ Maximum average roll value.

Material Specification 522—Aggregates for Portland Cement Concrete

1. Scope

This specification covers the quality of fine aggregate and coarse aggregate for use in the manufacture of portland cement concrete.

2. Quality

Aggregate shall conform to the requirements of ASTM Specification C 33 for the specified sizes. Aggregates that fail to meet any requirement may be accepted only when either:

- a. The specified alternate conditions of acceptance can be proven before the aggregates are used on the job and within a period such that no work under the contract will be delayed by the requirements of such proof,
- or
- b. The specification for concrete expressly contains a provision of special mix requirements to compensate for the effects of the deficiencies.

3. Reactivity with alkalis

The potential reactivity of aggregates with the alkalis in cement shall be evaluated by petrographic examination and, where applicable, the chemical method of test, ASTM Designation C 289, or by the results of previous tests or service records of concrete made from similar aggregates from the same source. The standards for evaluating potential reactivity shall be as described in ASTM Specification C 33, appendix A1.

Aggregates indicated by any of the above to be potentially reactive shall not be used except under one of the following conditions:

- a. Applicable test results of mortar bar tests made according to ASTM Method C 227 are available which indicate an expansion of less than 0.10 percent at 6 months in mortar bars made with cement containing not less than 0.8 percent alkalis expressed as sodium oxide; or
- b. Concrete made from similar aggregates from the same source has been demonstrated to be sound after 3 years or more of service under conditions of exposure to moisture and weather similar to those anticipated for the concrete under these specifications.

Aggregates indicated to be potentially reactive, but within acceptable limits as determined by mortar bar test results or service records, shall be used only with low alkali cement, containing less than 0.60 percent alkalis expressed as sodium oxide.

4. Storing and handling

Aggregates of each class and size shall be stored and handled by methods that prevent segregation of particles sizes or contamination by intermixing with other material.

Material Specification 523—Rock for Riprap

1. Scope

This specification covers the quality of rock to be used in the construction of rock riprap.

2. Quality

Individual rock fragments shall be dense, sound, and free from cracks, seams, and other defects conducive to accelerated weathering. Except as otherwise specified, the rock fragments shall be angular to subrounded. The least dimension of an individual rock fragment shall be not less than one-third the greatest dimension of the fragment. ASTM D 4992 provides guidance on selecting rock from a source.

Except as otherwise provided, the rock shall be tested and shall have the following properties:

Rock type 1

- **Bulk specific gravity (saturated surface-dry basis)**—Not less than 2.5 when tested in accordance with ASTM C 127 on samples prepared as described for soundness testing.
- **Absorption**—Not more than 2 percent when tested in accordance with ASTM C 127 on samples prepared as described for soundness testing.
- **Soundness**—The weight loss in 5 cycles shall not be more than 10 percent when sodium sulfate is used or more than 15 percent when magnesium sulfate is used.

Rock type 2

- **Bulk specific gravity (saturated surface-dry basis)**—Not less than 2.5 when tested in accordance with ASTM C 127 on samples prepared as described for soundness testing.
- **Absorption**—Not more than 2 percent when tested in accordance with ASTM C 127 on samples prepared as described for soundness testing.

- **Soundness**—The weight loss in 5 cycles shall be not more than 20 percent when sodium sulfate is used or more than 25 percent when magnesium sulfate is used.

Rock type 3

- **Bulk specific gravity (saturated surface-dry basis)**—Not less than 2.3 when tested in accordance with ASTM C 127 on samples prepared as described for soundness testing.
- **Absorption**—Not more than 4 percent when tested in accordance with ASTM C 127 on samples prepared as described for soundness testing.
- **Soundness**—The weight loss in 5 cycles shall be not more than 20 percent when sodium sulfate is used or more than 25 percent when magnesium sulfate is used.

3. Methods of soundness testing

Rock cube soundness—The sodium or magnesium sulfate soundness test for all rock types (1, 2, or 3) shall be performed on a test sample of $5,000 \pm 300$ grams of rock fragments, reasonably uniform in size and cubical in shape, and weighing, after sampling, about 100 grams each. They shall be obtained from rock samples that are representative of the total rock mass, as noted in ASTM D 4992, and that have been sawed into slabs as described in ASTM D 5121. The samples shall further be reduced in size by sawing the slabs into cubical blocks. The thickness of the slabs and the size of the sawed fragments shall be determined by the size of the available test apparatus and as necessary to provide, after sawing, the approximate 100-gram samples. The cubes shall undergo five cycles of soundness testing in accordance with ASTM C 88.

Internal defects may cause some of the cubes to break during the sawing process or during the initial soaking period. Do not test any of the cubes that break during this preparatory process. Such breakage, including an approximation of the percentage of cubes that break, shall be noted in the test report.

Material Specification 523 Rock for Riprap (continued)

After the sample has been dried following completion of the final test cycle and washed to remove the sodium sulfate or magnesium sulfate, the loss of weight shall be determined by subtracting from the original weight of the sample the final weight of all fragments that have not broken into three or more fragments.

The test report shall show the percentage loss of the weight and the results of the qualitative examination.

Rock slab soundness—When specified, the rock shall also be tested in accordance with ASTM D 5240. Deterioration of more than 25 percent of the number

of blocks shall be cause for rejection of rock from this source. Rock shall also meet the requirements for average percent weight loss stated below.

- For projects located north of the Number 20 Freeze-Thaw Severity Index Isoline (fig. 523–1). Unless otherwise specified, the average percent weight loss for Rock Type 1 shall not exceed 20 percent when sodium sulfate is used or 25 percent when magnesium sulfate is used. For Rock Types 2 and 3, the average percent weight loss shall not exceed 25 percent for sodium sulfate soundness or 30 percent for magnesium sulfate soundness.
- For projects located south of the Number 20 Freeze-Thaw Severity Index Isoline, unless otherwise specified, the average percent weight loss for Rock Type 1 shall not exceed 30 per-

Figure 523–1 Number 20 freeze-thaw severity index isoline (map approximates the map in ASTM D 5312)



Material Specification 523 Rock for Riprap (continued)

cent when sodium sulfate is used or 38 percent when magnesium sulfate is used. For Rock Types 2 and 3, the average percent weight loss shall not exceed 38 percent for sodium sulfate soundness or 45 percent for magnesium sulfate soundness.

4. Field durability inspection

Rock that fails to meet the material requirements stated above (if specified), may be accepted only if similar rock from the same source has been demonstrated to be sound after 5 years or more of service under conditions of weather, wetting and drying, and erosive forces similar to those anticipated for the rock to be installed under this specification.

A rock source may be rejected if the rock from that source deteriorates in 3 to 5 years under similar use and exposure conditions expected for the rock to be installed under this specification, even though it meets the testing requirements stated above.

Deterioration is defined as the loss of more than one-quarter of the original rock volume, or severe cracking that would cause a block to split. Measurements of deterioration are taken from linear or surface area particle counts to determine the percentage of deteriorated blocks. Deterioration of more than 25 percent of the pieces shall be cause for rejection of rock from the source.

5. Grading

The rock shall conform to the specified grading limits after it has been placed within the matrix of the rock riprap. Grading tests shall be performed, as necessary, according to ASTM D 5519, Method A, B, or C, as applicable.

Material Specification 532—Supplementary Cementitious Materials

1. Scope

This specification covers the quality of supplementary cementitious materials for concrete.

2. Quality

Fly ash used as a partial substitution of Portland cement shall conform to the requirements of ASTM C 618, Class C or F except the loss on ignition shall not exceed 3 percent, unless otherwise specified. Lot-to-lot variation in the loss on ignition shall not exceed 1 percent. When specified, fly ash shall conform to one or more of the supplementary optional physical requirements listed in ASTM C618.

Blast-furnace slag used as a partial substitution of portland cement shall conform to ASTM Standard C 989 for ground granulated blast-furnace slag.

Material Specification 535—Preformed Expansion Joint Filler

1. Scope

This specification covers the quality of preformed expansion joint fillers for concrete.

2. Quality

Preformed expansion joint filler shall conform to the requirements of ASTM Specification D 1752, Type I, Type II, or Type III, unless bituminous type is specified. Bituminous type preformed expansion joint filler shall conform to the requirements of ASTM Specification D 994, or D 1751.

Material Specification 536—Sealing Compound for Joints for Concrete and Concrete Pipe

1. Scope

This specification covers the quality of sealing compound for filling joints in concrete pipe and concrete structures.

2. Type

The compound shall be a cold-application material unless otherwise specified and shall be a single component or multiple component type.

3. Quality

The sealing compound shall conform to the requirements of one of the following specifications:

- ASTM Specification C 990—Joints for concrete pipe, manholes, and precast box sections using preformed flexible joint sealants.
- ASTM Specification C 877—External sealing bands for noncircular concrete sewer, storm drain, and culvert pipe.

- ASTM Specification D 6690—Standard Specification for Joint and Crack Sealants, Hot Applied, for Concrete and Asphalt Pavements.
- ASTM Specification C 920—Elastomeric joint sealants for cold applied sealing and caulking of joints on mortar and concrete structures not subject to fuel spills. Use type S or M, grade NS for vertical joints; type S or M, grade P or NS for horizontal joints. For class 25, use M, quality materials shall be used for both vertical and horizontal joints unless otherwise specified.

The sealing compound if used with other joint material, such as fillers or gaskets, shall be compatible.

Material Specification 582—Galvanizing

1. Scope

This specification covers the quality of zinc coatings applied to iron and steel productions.

2. Quality

Zinc coatings shall conform to the requirements of ASTM A 123 for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Products or as otherwise specified in the items of work and construction details of the Construction Specification.

ASTM A 123 covers both fabricated and nonfabricated products; e.g., assembled steel products, structural steel fabrications, large tubes already bent or welded

before galvanizing, and wire work fabricated from noncoated steel wire. It also covers steel forgings and iron castings incorporated into pieces fabricated before galvanizing or which are too large to be centrifuged (or otherwise handled to remove excess galvanizing bath metal).

Items to be centrifuged or otherwise handled to remove excess zinc shall meet the requirements of ASTM A 153, except bolts, screws, and other fasteners 0.5 inch or less in diameter may be coated with electro-deposited zinc or cadmium coating conforming to the requirements of ASTM B 766, coating thickness Class 5, Type III, or ASTM B 633, Service Condition SC-3, unless otherwise specified.

Material Specification 592—Geotextile

1. Scope

This specification covers the quality of geotextiles.

2. General requirements

Fibers (threads and yarns) used in the manufacture of geotextile shall consist of synthetic polymers composed of a minimum of 85 percent by weight polypropylenes, polyesters, polyamides, polyethylene, polyolefins, or polyvinylidene-chlorides. They shall be formed into a stable network of filaments or yarns retaining dimensional stability relative to each other. The geo-textile shall be free of defects and conform to the physical requirements in tables 592-1 and 592-2. The geotextile shall be free of any chemical treatment or coating that significantly reduces its porosity. Fibers shall contain stabilizers and/or inhibitors to enhance resistance to ultraviolet light.

Thread used for factory or field sewing shall be of contrasting color to the fabric and made of high strength polypropylene, polyester, or polyamide thread. Thread shall be as resistant to ultraviolet light as the geotextile being sewn.

3. Classification

Geotextiles shall be classified based on the method used to place the threads or yarns forming the fabric. The geotextiles will be grouped into woven and nonwoven types.

Woven—Fabrics formed by the uniform and regular interweaving of the threads or yarns in two directions. Woven fabrics shall be manufactured from monofilament yarn formed into a uniform pattern with distinct and measurable openings, retaining their position relative to each other. The edges of fabric shall be selvaged or otherwise finished to prevent the outer yarn from unraveling.

Nonwoven—Fabrics formed by a random placement of threads in a mat and bonded by heat-bonding, resin-bonding, or needle punching. Nonwoven fabrics shall be manufactured from individual fibers formed into a random pattern with distinct, but variable small openings, retaining their position relative to each other when bonded by needle punching, heat, or resin bonding. The use of nonwovens other than the needle punched geotextiles is somewhat restricted (see note 3 of table 592-2).

4. Sampling and testing

The geotextile shall meet the specified requirements (table 592-1 or 592-2) for the product style shown on the label. Product properties as listed in the latest edition of the "Specifiers Guide," Geosynthetics, (Industrial Fabrics Association International, 1801 County Road B, West Roseville, MN 55113-4061 or at <http://www.geosindex.com>) and that represent minimum average roll values, are acceptable documentation that the product style meets the requirements of these specifications.

For products that do not appear in the above directory or do not have minimum average roll values listed, typical test data from the identified production run of the geotextile will be required for each of the specified tests (tables 592-1 or 592-2) as covered under clause AGAR 452.236-76.

5. Shipping and storage

The geotextile shall be shipped/transported in rolls wrapped with a cover for protection from moisture, dust, dirt, debris, and ultraviolet light. The cover shall be maintained undisturbed to the maximum extent possible before placement.

Each roll of geotextile shall be labeled or tagged to clearly identify the brand, class, and the individual production run in accordance with ASTM D 4873.

Material Specification 592 Geotextile (continued)**Table 592-1** Requirements for woven geotextiles

| Property | Test method | Class I | Class II & III | Class IV |
|---|--------------------------------|--|--|--|
| Tensile strength (pounds) ^{1/} | ASTM D 4632 grab test | 200 minimum in any principal direction | 120 minimum in any principal direction | 180 minimum in any principal direction |
| Elongation at failure (percent) ^{1/} | ASTM D 4632 grab test | <50 | <50 | <50 |
| Puncture (pounds) ^{1/} | ASTM D 4833 | 90 minimum | 60 minimum | 60 minimum |
| Ultraviolet light (% residual tensile strength) | ASTM D 4355 150-hr exposure | 70 minimum | 70 minimum | 70 minimum |
| Apparent opening size (AOS) | ASTM D 4751 | As specified, but no smaller than 0.212 mm (#70) ^{2/} | As specified, but no smaller than 0.212 mm (#70) ^{2/} | As specified, but no smaller than 0.212 mm (#70) ^{2/} |
| Percent open area (percent) | CWO-02215-86 | 4.0 minimum | 4.0 minimum | 1.0 minimum |
| Permittivity sec ⁻¹ | ASTM D 4491 | 0.10 minimum | 0.10 minimum | 0.10 minimum |

^{1/} Minimum average roll value (weakest principal direction).

^{2/} U.S. standard sieve size.

Note: CWO is a USACE reference.

Material Specification 592 Geotextile (continued)**Table 592-2** Requirements for woven geotextiles

| Property | Test method | Class I | Class II | Class III | Class IV ^{3/} |
|---|--------------------------------|--|--|--|--|
| Tensile strength (lb) ^{1/} | ASTM D 4632 grab test | 180 minimum | 120 minimum | 90 minimum | 115 minimum |
| Elongation at failure(%) ^{1/} | ASTM D 4632 | ≥ 50 | ≥ 50 | ≥ 50 | ≥ 50 |
| Puncture (pounds) | ASTM D 4833 | 80 minimum | 60 minimum | 40 minimum | 40 minimum |
| Ultraviolet light (% residual tensile strength) | ASTM D 4355 150-hr exposure | 70 minimum | 70 minimum | 70 minimum | 70 minimum |
| Apparent opening size (AOS) | ASTM D 4751 | As specified max. #40 ^{2/} | As specified max. #40 ^{2/} | As specified max. #40 ^{2/} | As specified max. #40 ^{2/} |
| Permittivity sec ⁻¹ | ASTM D 4491 | 0.70 minimum | 0.70 minimum | 0.70 minimum | 0.10 minimum |

1/ Minimum average roll value (weakest principal direction).

2/ U.S. standard sieve size.

3/ Heat-bonded or resin-bonded geotextile may be used for classes III and IV. They are particularly well suited to class IV. Needle-punched geotextiles are required for all other classes.

SPECIAL PROVISIONS
WQI WETLAND PROJECT CAL883416C
CALHOUN COUNTY, IOWA
2024

PART 1: GENERAL

1.1 Scope

The basis of design for the lift station is pump model DS_6NNT by Cornell Pump Company, Clackamas, Oregon. The lift station shall be capable of providing a minimum of 1350 gpm at 22.3' TDH. Bids will be evaluated on cost and compatibility with design/specifications.

Any bids proposing lift station equipment other than manufactured by Cornell Pump, Inc. stated above, shall submit a proposal package with their bids to show they are equivalent and compatible with design/specifications. The bid package shall clearly identify specific information pertaining to the proposed equipment including performance, materials, dimensions, hp, controls, etc. for each of the items included in the package that are specified here in this specification.

PART 2: MATERIALS

2.1 Submersible Pumps

The Contractor shall furnish and install two (2) submersible pumps and apparatus, which meet the following criteria:

| | |
|------------------------------|-----------------------|
| Rated Pump Speed | 1,800 RPM (Max) |
| Capacity at Rated Speed | 1,350 GPM @ 22.3' TDH |
| Allowable HP | 15 HP |
| Minimum Test Sphere Diameter | 3 Inch |
| Discharge Outlet Diameter | 6 Inch |

The pump manufacturer shall submit a dimensional installation layout drawn to scale for approval with all necessary pump data and performance curves before filling the order for the pumps. Matching components shall be submitted for approval by the Engineer. Any changes to what is shown on the plans that are required for pump installation shall be submitted to the Engineer for approval and shall be incidental to the project. The pumps shall be manufactured by Cornell, or pre-approved equal.

Pumps shall be capable of handling unscreened stream water. The design shall be such that pumping units will be automatically connected to the discharge piping when lowered into place on the discharge connection. The pumps shall be easily removable for inspection or service, requiring no bolts, nuts, or other fastenings to be removed for this purpose, and no need for personnel to enter pump well. Each pump

shall be fitted with a stainless steel cable of adequate strength and length to permit raising the pump for inspection and removal.

The stator casing and oil casing shall be cast iron. Impeller shall be of hard-iron or flame hardened iron construction. All external bolts and nuts shall be of stainless steel. The common motor and pump shaft shall be stainless steel and equipped with bronze bushings. Shafts shall run in two single rows with permanently lubricated ball bearings designed for 40,000 hours of operation. The impeller shall be protected by a bronze wear ring and neoprene O-ring at the inlet of the pump. The impeller shall be of multi-vane closed, non-clog design, capable of passing solids, and fibrous material, and constructed with long throughway with no acute turns.

Each pump shall be provided with a tandem double mechanical seal running in an oil reservoir, composed of two separate lapped face seals, each consisting of one stationary and one rotating silicon carbide ring with each pair held in contact by a separate stainless steel spring.

A sliding guide bracket shall be an integral part of the pumping unit and the pump casing shall have a machined angle connection with yoke to connect with the cast iron discharge connection, which shall be bolted to the floor of the sump and so designed as to receive the pump angle connection without the need for any bolts or nuts.

Pump motors shall be 460 Volt, 3 Phase, 60Hz and equipped with high temperature thermal cut-out probes and controls, pump seal leak detection system, durable flexible power cords of adequate length and corrosion resistant water and moisture tight seals, all provided by pump supplier. The pump motors shall be explosion proof, submersible, induction type inverter duty rated with a squirrel cage rotor, shell type design, housed in an air filled watertight chamber. Pump motor shall be designed for a minimum of 15 equally spaced starts per hour and the service factor shall be a minimum of 1.15.

The Contractor shall warrant the pump being supplied to the Owner against defects in workmanship and materials for a period of five (5) years under normal use, operation and service. The warranty shall be in published form and apply to all similar units. A copy of this warranty shall be placed on file with the Owner and Engineer prior to installation of this equipment.

2.2 Electrical

All electrical devices called for in these plans and specifications shall be furnished and installed and connected for operation by the Contractor. Three phase, 480 volt power will be available at the site where shown on the plans. Contractor shall extend service from that point. Electrical devices shall meet the requirements specified herein.

2.3 Supervisory and Pump Controls

Installation of the controls and all electrical work and materials for connection of the power to the controls and shall be provided by the Contractor's electrician.

Pumps shall operate so only one pump runs at a time, and they alternate duty on every pump cycle.

Pump control panel shall accept a 277/480 volt, 200 amp, three phase, four wire, power supply and include main disconnects for utility and generator feeds, lockable in the OFF position, as well as a 200 amp automatic transfer switch. Control panel shall be service entrance rated. Panel shall include provisions for power distribution to wet well pumps and other internal and external ancillary loads identified on drawings and required for proper operation. Provide all required circuit breakers, disconnects, fuses, and terminals. Main disconnects and lift pump circuit breakers shall be accessible from front of panel and lockable in the OFF position. Control panel shall be NEMA 3R stainless steel enclosure, minimum 12-gauge construction and include locking provisions. Control panel shall be freestanding on mounting feet to provide air gap between bottom of panel and ground for isolation of hazardous area conduits. Provide vented stainless steel covers to cover air gap on front and back of control panel to protect exposed cables. Control panel shall include any required heaters and ventilation equipment for operation in ambient conditions of -20 to 100 degrees F. Control panel shall include a switched light, interior to the panel, to illuminate panel interior for maintenance or troubleshooting activities as well as an interior ground fault interruption type receptacle.

Control panel shall include a 20 amp, 120 volt, convenience outlet. The convenience outlet shall be ground fault interruption type, weatherproof and accessible without opening enclosure.

Control panel shall be U.L. listed and labeled.

The control panel shall include the following, as a minimum.

- A. Circuit breakers for each circuit required for proper operation and power distribution. Circuit breakers supplying variable frequency drives shall be sized according to VFD size and to VFD manufacturer recommendations.
- B. Variable frequency drives for each motor, with over current, over temperature, and short circuit protective features. Variable frequency drives shall be sized to provide output rating equal to pump motors. When called to operate, variable frequency drive shall go to 100% rated speed (60 Hz output).
- C. Control power transformer, sized as required for expected loads plus 25% spare capacity.
- D. Hand-Off-Automatic Selector Switch for each pump along with a green running light and red fail light for each pump.
- E. Pump monitor provided by pump manufacturer for seal failure and over temp with seal failure and high temperature red fail light for each pump.
- F. Green light for transfer switch on Normal Source. Red light for transfer switch on Emergency Alternate Source.

- G. Liquid level regulator controller assembly with automatic pump operation, and electrical alternation.
- H. Pump running time meter for each pump and a third meter recording when the two pumps are running simultaneously, reading in hours with one decimal.
- I. A red alarm light, mounted on top of the panel, to allow indication of an alarm from a distance. Light shall actuate for any alarm condition. Light bulb shall be LED type, with light output equivalent to a 100 watt incandescent bulb.
- J. Supervisory monitoring/telemetry, see proposal from Automatic Systems Company included at the end of this specification for reference.
- K. The following alarms, as a minimum, shall be made available for connection to the monitoring equipment:
 - 1. Utility Power Fail
 - 2. Control System Fail
 - 3. Generator Fail
 - 4. Transfer Switch on Emergency
 - 5. Pump 1 Seal Fail
 - 6. Pump 1 High Temp
 - 7. Pump 2 Seal Fail
 - 8. Pump 2 High Temp
 - 9. High Wet Well Level
 - 10. Low Wet Well Level
- L. Terminals shall be provided for connection of the electric power supply wires, circuit wires, instrumentation wires, and other electrical connections, and shall be coded.
- M. Control panel to include all other relays, wiring, terminals, and other ancillary items required for proper operation.

2.4 Level Regulators

Install a complete duplex lift station level control lift station system. Level control system shall consist of one (1) submersible level transmitter and two (2) liquid level backup float switches. Level transmitter and float switches shall be installed and anchored securely per Manufacturer's recommendations.

Transmitter shall be constructed of type 316 stainless steel housing and contain a solid state semiconductor sensor and Hastelloy C diaphragm. Transmitter shall measure depths from 0 to 20 feet and output a 4-20 mA signal in proportion to level. Accuracy shall be plus or minus 0.5% of full scale. Transmitter shall include integral cable of adequate length and all accessories required for mounting of transmitter. Level transmitter shall be by Ametek U.S. Gauge Model 575P or equal.

Each float switch shall consist of a conical part and a hemispherical bottom covered with a hard PVC casing, all enclosing an eccentrically fitted lead-free weight to keep the regulator in the correct attitude, and shock proof mercury-free switch. Float switch

shall have a 1" maximum differential. The float switches shall be used to indicate a low and high alarm condition. Floats switches shall be installed and anchored securely per Manufacturer's recommendations.

2.5 Lift Station Structure

The Contractor shall furnish and install precast reinforced manhole sections and monolithic base which meet SUDAS specifications for sizes shown on plans. All necessary block outs for entrance/exit pipes, etc. shall be formed at the proper location when the section is cast. All joints shall be fitted with a neoprene rubber gasket to ensure water tightness and each joint also shall be sealed with an approved bituminous joint sealer and seal wraps.

2.6 Pump Guide Rails

The Contractor shall furnish and install dual guide rails for each pump, to permit smooth raising and lowering of the pumps for the entire length without any problems. Guide rails shall be stainless steel pipe of adequate size and length to extend from the lower guide holders on the pump discharge connections to the upper guide holders. Contractor shall install guide rails in accordance with the manufacturer's instructions.

2.7 Access Doors

All access doors shall have an aluminum frame properly cast in concrete cover. Hatches shall have torsion hinged and hasp equipped cover, spring assist hinges, and a safety locking handle in the open position. The doors shall be ¼" diamond pattern aluminum rated for AASHTO H20 highway loading. Hardware shall be 316 stainless steel throughout. Access doors shall include fall safety grating or fall protection. Fall protection nets will not be accepted. Provide Halliday Products series H2R or equal for the wet well.

The lift station wet well frame shall include upper guide holder and level sensor cable holder. Frame shall be properly located above the pumps to provide minimum clear opening shown on plans sheets. The cover shall be the double leaf type.

2.8 Vent

A suitable schedule 40 return bend vent with aluminum bird screen shall be furnished and installed as shown on the drawings.

2.9 Piping and Fittings

1. This work consists of the construction of HDPE Pressure Pipe, Ductile Iron Pipe and fittings as shown in the plans.
2. Inlet piping shall be HDPE high density polyethylene (HDPE) pressure pipe.
 - a. Pipe shall be joined by means of zero leak-rate butt fusion and shall conform with ASTM D3350. Materials used for manufacturing of polyethylene pipe and fittings shall be PE3408 High Density

Polyethylene meeting the ASTM D3350 cell classification of 345434C. Polyethylene pipe shall be manufactured in accordance with ASTM F-714. The pressure rating shall be 160 psi and the standard dimension ratio shall be SDR11 unless otherwise called for in the plans. Pipe diameter (O.D.) sizes shall be ductile iron pipe sizes (DIPS).

- b. Polyethylene fittings shall be made from material meeting the same requirements as the pipe. Butt fusion fittings shall comply with ASTM D3261 requirements. All fittings and the pipes shall be supplied by the same manufacturer to insure compatibility to polyethylene resins.
- c. Polyethylene pipe shall be joined by the butt fusion method which shall be performed in strict accordance with the recommendations of the pipe manufacturer. Equipment used for butt fusion shall be capable of meeting all requirements recommended by the pipe manufacturer, including but not limited to alignment, temperature, and fusion pressures.

3. Discharge Piping shall be Ductile Iron Pipe.

- a. Pipe shall be Class 53 Ductile Iron Pipe manufactured in accordance with AWWA C111 and C151/A21.51 standard specification for centrifugally cast ductile iron pipe for water or other liquids.
- b. Connections for Ductile Iron Pipe and fittings shall be flanged or if supported by soil, connections shall be mechanical joint with retaining glands. Tapping ports shall be installed into the piping assembly to allow for air release and accept gauges for pressure readings of the pumps. The pipes shall be sealed where they protrude through the base plate with link seal to form a gas tight seal between the pump chamber and wet well.
- c. Ductile iron pipe and fittings shall be furnished with standard thickness cement interior coating conforming to ANSI/AWWA C104. Pipe used for buried locations shall be furnished with standard bituminous exterior coating. Pipe used for interior and exposed locations shall be furnished without coating to facilitate painting. All mill scale, rust, weld flux and other foreign matter shall be removed from all steel surfaces by shot blasting to SSPC SP-10 specification for near-white blast cleaning. Surface irregularities shall be removed by grinding. The piping and control panel bracket surfaces shall receive hi-build epoxy coating per the painting schedule listed in part 2.20 of this specification. Paint touch-up shall be provided as necessary for damaged coating areas.
- d. All plugs, caps, tees, and bends for Ductile Iron Piping deflecting 22-1/2 deg. or more on mains 6 inches in diameter or larger shall be provided with a restrained joint.
- e. Mechanical joint retainer glands for Ductile Iron Pipe shall be Megalug Retainer Glands as manufactured by EBBA Iron, Inc. or approved equal.

- f. All fittings, valves, hydrants, etc. for Ductile Iron Pipe shall be installed with Stainless Steel T-bots as manufactured by Birmingham Fasteners or an approved equal
- g. Pipe supports shall be Grinnell figure no. 264 or equal.
- h. Process piping associated with the lift station may be field tested for leakage at 150 psi as described elsewhere in these specifications.
- i. Insulation shall be provided as noted on the plans.
 - i. Insulation shall be 1" thick EPDM (NBR/PVC blend not permitted) elastomeric cellular foam. ANSI/ASTM C534, flexible plastic, 0.25 maximum 'k' value at 75 degrees F, 25/50 flame spread/smoke developed rating when tested in accordance with ASTM E84 (UL 720). Insulation shall be John Mansville or equal.
 - ii. Cover the insulation with plastic jacket covering. Position seams to shed water. Plastic jackets and fitting covers shall be high impact, glossy white, 0.030" thick, self-extinguishing plastic. Cover shall be suitable for use indoors or outdoors with ultraviolet inhibitors, suitable for -40 degrees F to 150 degrees F, and with 25/50 maximum flame spread/smoke developed. Jacketing shall be John Mansville or equal.
 - iii. Install insulation after piping has been tested. Pipe shall be clean, dry and free of rust before applying insulation.
 - iv. Solvent weld all joints with manufacturer recommended cement. Overlap all laps and butt joints 1.5" minimum. Repair loose ends that do not seal securely. Solvent weld all fittings covers in the same manner. Final installation shall be watertight.

4. Plug Valves

- a. Plug valves shall be of the non-lubricated, eccentric type with resilient faced plugs and flanged or mechanical joint ends as shown on the plans. Port areas of valves shall be at least 80% of full pipe area. Valve bodies shall be pressure rated not less than 125 psi. Valve shall have operating nut and key with extension to operate valve nut from ground level. Bodies shall be cast iron with raised seats. Seats shall have a welded-in overlay of not less than 90% pure nickel on all surfaces contacting the plug face. Valves shall have stainless steel permanently lubricated upper and lower plug stem bushings. All valves 4" and larger shall be of the bolted bonnet design. Operating shaft seal on all valves shall be multiple ring "U" cup type or adjustable packing type. Shaft seals on valves 4" and larger shall be repairable without removing valve bonnet. All exposed nuts, bolts, springs and washers on all valves shall be zinc plated. Flanged valves

shall be furnished with ANSI/AWWA Standard 125 lb. flanges. Mechanical joint type ends shall conform to ANSI A21.11.

- b. Plug valves shall be as manufactured by Clow Corporation, Henry Pratt Company, Milliken Valve, Dezurik, or equal.

2.10 Site Work

The Contractor shall make excavations in such a manner as to protect existing property and structures. Sheeting and shoring shall be used as required, if necessary. Contractor shall leave the site in a neat and workmanlike condition with proper drainage maintained. The lift station shall be positioned on undisturbed soil and backfilled as noted on the drawings. Refer to Specification IA-23 for more details.

2.11 Painting

The Contractor shall furnish all labor, materials and equipment required to complete painting as indicated in the following schedule. All items shall be thoroughly cleaned before painting. Paint and paint products shall be manufactured by Tnemec Company Inc. or pre-approved equal. All piping and other metal shall be gray in color. Paint color of concrete shall be as selected by the Owner.

| | | <u>Tnemec</u> | <u>Thickness</u> |
|---|-------------------------------|----------------------------|------------------|
| Lift station interior concrete, pumps, piping, and conduit | Parge Coat (Concrete Only) | Series 218 Mortar Clad | 1/16"-1/4" |
| | 1st Coat | Series G435 Perma-Glaze | 20 mils |
| | 2nd Coat | Series G435 Perma-Glaze | 20 mils |

All metal shall be painted except for aluminum and stainless steel. Aluminum shall be coated with asphalt or approved coating where in contact with dissimilar materials.

2.12 Start-Up of Equipment

Coordinate installation and start-up scheduling with Owner and Engineer.

Pump equipment supplier shall provide skilled programmer/instrumentation engineer or technician who shall complete troubleshooting and start-up to place entire system into satisfactory operation; engineer or technician shall make necessary inspection of completed installation, make necessary final field adjustments and make program revisions as required for start-up.

Devices requiring field calibration shall be calibrated in the presence of Owner's representative and documented. Demonstrate proper operation of all system features and functions to Owner and Engineer.

END